



NO: 2020/3581

Dated: 11.06.2020

**TO WHOMSOEVER IT MAY CONCERN**

Following designs of Solar Module Mounting Structure submitted by M/S *Ethos Power Pvt. Ltd*

S. No.	Structure Table Size	Drawing No.
01	6 Modules Mounting Structure	SEPL/VT/P02202021/M01
02	9 Modules Mounting Structure	SEPL/VT/P02202021/M02

have been checked & evaluated using STAAD Pro report for various parameters as per IS 875 : 2015, IS 800 : 2007 for a wind speed of 155kmph and hence it is approved.

The above designs have been compared with "Indicative MNRE Design" of the tender no. NIT/Bid Document No. EESL/06/2019-20/KUSUM/SWPS/Phase-01/Off Grid/192007012 dated 21-08-2019 (Section 4, Annexure - A) and had been found to be improved alternative over "Indicative MNRE Design" as shown below:

S. No.	Feature	Improvement in ALTERNATE Design	Advantage of Improvement
01	Higher Wind Speed withstanding capacity	ALTERNATE Design can withstand a wind speed of 155kmph which is higher than the requirement of 150kmph by MNRE.	Withstanding Higher wind speed would result in a better reliability and longer life of the MMS.
02	All parts per BIS standards	All the members used in ALTERNATE Design are as per IS 1161 where as several members of MNRE Design are not conforming IS 1161. (Refer Annexure 01)	100% conformity to BIS would result in better quality
03	Suitability of Foundation for different soil types.	As per MNRE Annexure – A of the tender document, foundation is with base plate and direct piling design is used. The "ALTERNATE Design" has piling foundation and such type of foundation is better suited for all types of soils including soils having low bearing capacity.	<ul style="list-style-type: none"> <li>- The proposed foundation is more suitable for sandy soils</li> <li>- Pile cap of 200mm will ensure that there is no damage to MMS due to water logging.</li> </ul>

Certified By:

Prof. Khalid Moin  
 (Structural Engineer)

Professor

**Dr. KHALID MOIN**  
 Professor  
 Deptt. of Civil Engineering  
 FTO Engineering & Technology  
 Jamia Millia Islamia  
 New Delhi-110025

**DESIGN BASIS REPORT**  
**FOR**  
**6 MODULE SOLAR PUMP**  
**MOUNTING STEEL STRUCTURE TABLE**

For  
ETHOS POWER PVT LTD.  
121-123, 1st Floor, NH – 8 DLF Star Tower, Sector  
30, Gurugram, Haryana, India - 122001

Date: 06<sup>th</sup> JUNE 2020

Approved :-  
  
Dr. ISHAQID ALI  
Professor  
Dept. of Civil Engineering  
P.O. Engineering & Technology  
Jemai Mala Ishamia  
New Delhi-110025

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*Approved /*

Dr. ISMAILI STOW  
 Professor  
 Dept. of Civil Engineering  
 P.O Engineering & Technology  
 Jamia Millia Islamia  
 New Delhi - 110025

## **1. GENERAL:**

### **2.1 SCOPE**

This document covers the Analysis & Design of Fixed type Solar PV Module mounting structure at an angle of **tilt 25 degree** for at **155kmph** wind speed.

### **2.2 UNITS OF MEASUREMENTS**

Units of measurements used in analysis shall be of SI Units.

### **2.3 CODES AND REFERENCE**

Following have been referred for analysis and design of the MMS

Project Reference Drawings/Documents		
1	-----	Geo-Technical Investigation
2	-----	Plot Plan
3	-----	Module GA
Indian Standard Codes		
4	IS 456 - 2000	Plain and Reinforced Concrete - Code of Practice
5	IS 875: Part 1 & 2	Code of practice for the design loads for building and structures
6	IS 875: Part 3 - 2015	Code of practice for the design loads for building and structures - Wind Loads
7	IS 800: 2007 (WSD)	Code of practice for general construction in steel of hot rolled
8	IS 801: 1975	Code of practice for cold formed steel
9	IS 811: 1987	Specifications for cold formed light gauge structural steel sections
10	IS: 2062	Hot rolled medium and high tensile structural steel

Approved : -

Dr. IQBAL ID MOIN  
Professor  
Dept. of Civil Engineering  
T.O. Engineering & Technology  
Jaipal Mills Jorawar  
New Delhi-110025

## 2.4 MATERIAL PROPERTIES:

Material	Property	Value	Units
Concrete - M25	Density	25	kN/m <sup>3</sup>
	Characteristic Strength	25	N/m <sup>2</sup>
	Modulus of Elasticity	25000	N/m <sup>2</sup>
High Strength Reinforcing Steel	Density	78.5	kN/m <sup>3</sup>
	Characteristic Strength	500	N/m <sup>2</sup>
	Modulus of Elasticity	200000	N/m <sup>2</sup>
Structural Steel	Density	78.5	kN/m <sup>3</sup>
	Characteristic Strength	350000	kN/m <sup>2</sup>
	Modulus of Elasticity	250000	N/m <sup>2</sup>

## 2.5 PLANT INFORMATION:

Location of Site: India  
Basic Wind Speed: 43 m/s (155kmph)

## 2.6 SOFTWARE USED:

Below listed software are used for the structural analysis and design;

S. No.	Software Name	Developed By	Usages
1	STAAD Pro	Research Engineers Inc., Berkely, USA	Structural Analysis And Design
2	In-house developed MS -Excel Programs	.....	Load Calculations

Approved :-

Dr. JHALID MOIN  
Professor  
Dept. of Civil Engineering  
F/O Engineering & Technology  
Jamia Millia Islamia  
New Delhi-110025

## 2. LOADS:

### 2.1 Load Calculation:

Dead Load Calculation:-		UDL LOAD		
Module	=			
Solar module Dimension	=	Length (mm)	Width (mm)	Nos
		1964	986	6
Weight per module	=	22.5	Kg's/module	
Weight of 6 modules	=	135	Kg's	
Module Configuration	=	2X3	Portrait	
Including 10% extra Load per for connections	=	135	Kg's	
Purlin Length	=	3	m	
Total Length of purlin	=	12	m	
Udl load at purlin	=	0.110	kN/m	
All dead loads have been applied as udl load by considering the modules are in constant touch with the purlin upper face.				

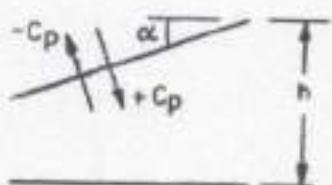
#### 2.1.1 Wind Load:

Wind Pressure Calculation				
Basic Wind Speed	=	60.00	Kmph	
Basic Wind Speed	=	42	m/sec	
Design Period	=	25	years	
Type of MMS	=	Ground Based		As per Tender Specification
Terrain Category	=	2		
Class	=	A		
Module Clearance From HSL	=	0.85	m	
		Length	Width	Nos
Solar module Dimension	=	1964	986	6
Fast Co-efficient k1	=	0.91		(As per Table-1, IS-875 part-3)
Terrain Factor, k2	=	1.00		For Terrain Category 2
Topography Factor k3	=	1.00		(As per Table-2 of IS-875 Part 3- 1987)
Importance Factor k4	=	1.00		(Refer IS-875 part-3 clause-6.3.4)
Design Wind Speed, Vd=19.2*1.294=	=	37.92	m/sec	
Design Wind pressure, Pd = 0.67(Vd)^2=	=	710.84	N/m^2	
As per Latest code IS : 875 (Part-3-2015)				
The design wind pressure pd can be obtained as as per clause 4.5.2 [pd = Kd . Ka . Kc . Pz]				
Where,				
Wind directionality factor kd	=	0.90		(Refer clause-7.2.1, In IS-875-part-3)
Area of solar panels	=	8.62	m^2	
Area averaging factor, ka	=	0.95		(if this area is > 25 than Kd = 0.9)
Combination factor kc	=	1		(Refer clause-7.3.3.13, In IS-875-part-3)
Design pressure shall be, pd = Kd . Ka . Kc . Pz=	=	621.08	N/m^2	

Approved :-

Dr. KHALID MOIN  
 Professor  
 Dept. of Civil Engineering  
 F/O Engineering & Technology  
 Jamia Millia Islamia  
 New Delhi-110025

## Calculating pressure coefficient for mono-slope roof;



### SIGN CONVENTION

+TOWARDS THE STRUCTURE (SUCTION), - AWAY FROM THE STRUCTURE (UPLIFT)

For calculating the force on the panels from the pressure calculated above, we need pressure coefficient. The current structure falls under mono-slope free roofs. Pressure coefficient for mono-slope roof is found out referring section 7.3.3.2, Table 8, IS: 875, Part 3 - 2015. The pressure coefficients for a mono-slope free roof of 05° tilt angle and solidity ratio of 0.

WIND LOAD SIDES A11 (X & Z DIRECTIONS)		SECTION DETAILS				
STRUCTURAL ELEMENTS		Web	Flange	Lip	Deck	
Columns	=	127	127	0	4.3	
Eccentric	=	100	60	11	0	
BRACING	=	33.7	33.7	0	2.0	
<i>As per table 10, IS-875 part 3</i>						
Design Wind speed, Vd	=	17.82	Vertical			
Design Wind pressure, Pd	=	421.00	Horizontal			
Surface		Angle of Wind	Cx	Wind facing width in mm(D) (X-direction)	Length of member in mm(E)	Drd. in m <sup>2</sup> /sec (X-direction)
Column	=	0°	0.56	127	1117	0.10
Eccentric	=	0°	1.00	100	3000	0.00
Surface		Angle of Wind	Cx	Wind facing width in mm(D) (Z-direction)	Length of member in mm(E)	Drd. in m <sup>2</sup> /sec (Z-direction)
Column	=	90°	0.10	127	1117	0.10
BRACING	=	90°	0.10	33.7	1170	0.10
Wind Force at Column per meter Fwd. width "a" x "b" =		0.0012	Flange		Z-direction	Reduction factor (K)
Wind Force at Eccentric per meter Fwd. width "a" x "b" =	=	0.1048	Flange			
Wind Force at Column per meter Fwd. width "a" x "b" =		0.0029	Flange		Z-direction	Reduction factor (K)
Wind Force at Bracing per meter Fwd. width "a" x "b" =	=	0.0068	Flange			

Approved :-

Dr. KHALID MOON  
Professor  
Dept. of Civil Engineering  
P.O. Engineering & Technology  
Jemia Millia Islamic  
New Delhi-110026

WIND LOAD UPWARD-DOWNWARD (+ Y DIRECTION)				
Solar panel Dimensions	=	Length 1000	Width 900	Nos 0
Tilt angle of solar panel	=	25	Degree	
Upward Downward Wind Force, $F = Cp * A * Pd$ Where,				
Pressure Coefficient for Upward wind, $Cp$	=	-1.6		(As per IS875 Part-3, Table-8)
Surface Area of Immersed element $A = (1000 \times 900) / (10^3 \cdot 10^6)$	=	11.62	sqm	
Design wind pressure, $Pd$	=	621.06	N/sqm	
Upward Wind force, $F_u = Cp * A * Pd$	=	-11.55	KN	
Pressure coefficient for Downward wind, $Cd$	=	1.00		(As per IS875 Part-3, Table-8)
Downward Wind force, $F_d = Cd * A * Pd$	=	7.22	KN	
Here, we are considering split wind force of solar panel shall be bear by Purlin				
No. of Purlin support	=	4	Nos	
Length of member	=	3	m	
Upward Wind force, $F_u = F_u / 4^{th}$	=	-0.96	KNm	
Downward Wind force, $F_d = F_d / 4^{th}$	=	0.601	KNm	

Where, Negative sign shows Upward direction of Wind

## 2.2 LOAD COMBINATION

LOAD TYPE	L/C	LOAD NAME
Primary	1	DEAD LOAD STRUCTURE
Primary	2	WIND LOAD X+
Primary	3	WIND LOAD X-
Primary	4	WIND LOAD Z+ UPWARD
Primary	5	WIND LOAD Z- DOWNWARD
Combination	6	1 DEAD LOAD + 1 WIND LOAD X+
Combination	7	1 DEAD LOAD + 1 WIND LOAD X-
Combination	8	1 DEAD LOAD + 1 WIND LOAD Z+ UPWARD
Combination	9	1 DEAD LOAD + 1 WIND LOAD Z- DOWNWARD
Combination	10	1 DEAD LOAD + 1 WIND LOAD X+ + 1 WIND LOAD Z+ UPWARD
Combination	11	1 DEAD LOAD + 1 WIND LOAD X+ + 1 WIND LOAD Z- DOWNWARD
Combination	12	1 DEAD LOAD + 1 WIND LOAD X- + 1 WIND LOAD Z+ UPWARD
Combination	13	1 DEAD LOAD + 1 WIND LOAD X- + 1 WIND LOAD Z- DOWNWARD

Approved :-

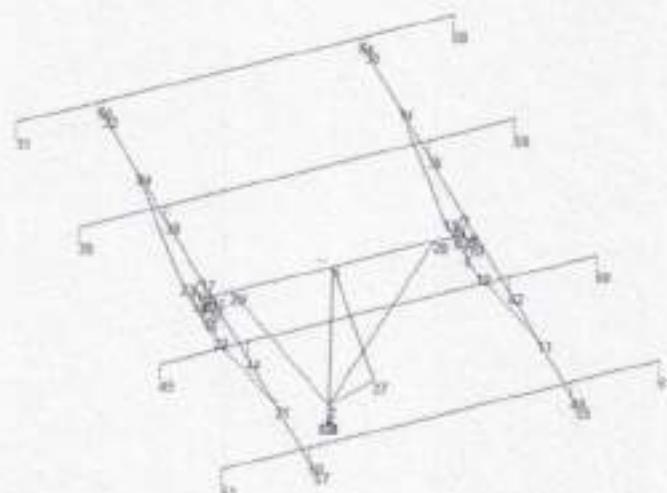


Dr. KHALID MOIN  
Professor  
Dept. of Civil Engineering  
FIC Engineering & Technology  
Jamia Millia Islamia  
New Delhi-110025

### 3. STAAD MODEL:

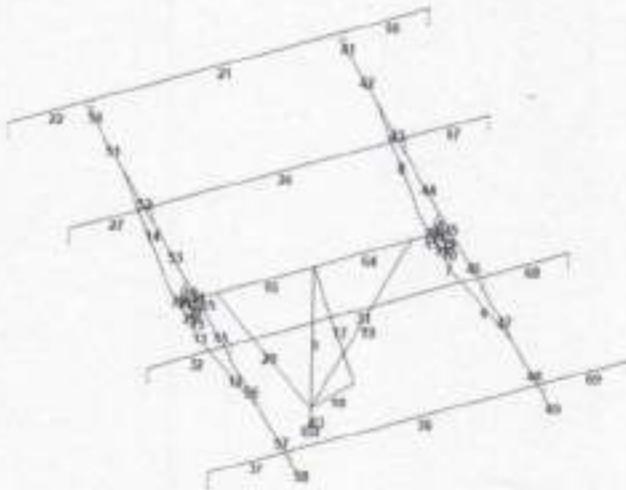


STAAD 3D Rendered View



NODE NUMBER

Approved :-  
  
Dr. KHALID MOIH  
Professor  
Dept. of Civil Engineering  
FIC Engineering & Technology  
Jama Millia Islamia  
New Delhi-110025



BEAM NUMBER

#### 4. DEFLECTION CHECK:

As per IS 800:2007, following table can be referred for deflection check.

Table 6-Deflection Limits

Type of Building	Deflection	Design Load	Material	Supporting	Maximum Deflection
(1)	(2)	(3)	(4)	(5)	(6)
Industrial Buildings	Steel	Live load/Wind load	Plates and Girders	Elastic cracking	Span/150
		Wind load	Simple span	Brittle cracking	Span/100
		Live load	Concentric span	Elastic cracking	Span/150
		Live load/Wind load	Reflex supporting	Elastic cracking	Span/150
		Cross load (Manual operation)	Gassy	Crack	Span/150
	Concrete	Cross load (Electrical operation up to 500A)	Gassy	Crack	Span/150
		Cross load (Electrical operation over 500A)	Gassy	Elastic cracking	Span/100
		Wind load	Concentric	Wind/Water	Height/150
		Cross + wind	Gassy (steel)	Crack (allowable)	Height/100
		Cross + wind	Concrete/Steel	Relative displacement between two supporting zones	10 mm
Other Buildings	Steel	Live load	Flat and Bush	Gassy/Elastic cracking: plastic moment	Height/150
		Live load	Concentric	Gassy/Elastic cracking: con. moment	Height/100
	Wind	Wind	Building	Elastic not acceptable in cracking	Span/150
		Wind	Wind energy diff.	Elastic cracking	Span/150

Approved by  
Dr. KHALID KHAN  
Professor  
Dept. of Civil Engineering  
FIR Engineering & Technology  
Jamia Millia Islamia  
New Delhi-110025

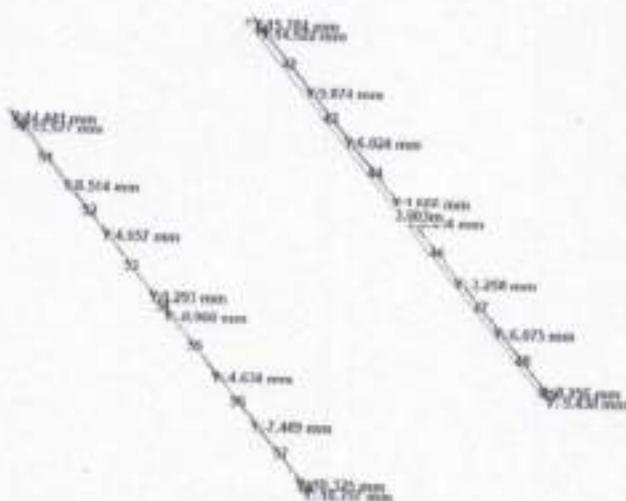
#### **Column deflection check:**



Maximum deflection in Column = 3.592 mm

Permissible limit for deflection in Column =  $1117 / 240 = 4.65$  mm > 3.592. Hence OK.

#### Rafter deflection check:

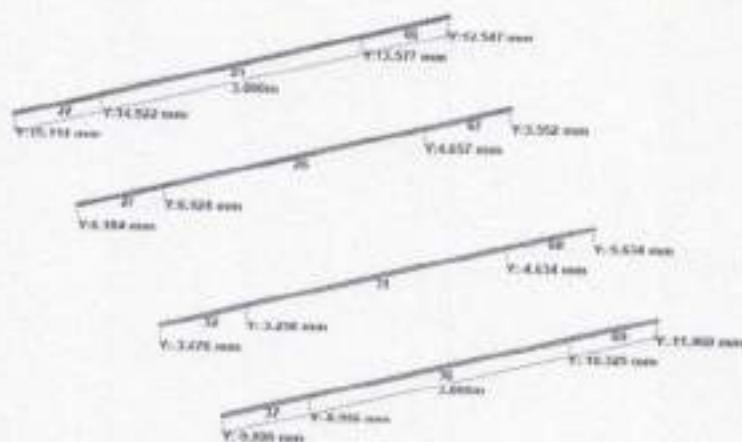


Maximum deflection in Rafter = 15.784 mm

Permissible limit for deflection in Rafter =  $3000 / 180 = 16.667 \text{ mm} \approx 15.784 \text{ mm}$  or  $1.58 \text{ cm}$

**Approved :-**  
  
Dr. IQBAL MOIN  
Professor  
Dept. of Civil Engineering  
FID Engineering & Technology  
Janta Mills Isamia  
New Delhi-110035

### Purlin deflection check:



Maximum deflection in Purlin = 14.922 mm

Permissible limit for deflection in Purlin =  $3000 / 180 = 16.66\text{mm} > 14.922$ , Hence OK

### 5. REACTION SUMMARY:

Node	UC	Horizontal	Vertical	Horizontal	Moment			
		Fx kN	Fy kN	Fz kN	Mx kN-m	My kN-m	Mz kN-m	
3	1 DEAD LOA	-0.000	2.274	-0.000	-0.105	0.000	-0.000	
	2 WIND LOA	-1.201	0.000	0.000	0.000	-0.028	1.378	
	3 WIND LOA	1.201	-0.000	-0.000	-0.000	0.028	-1.378	
	4 WIND LOA	-0.000	-10.441	-4.926	-5.905	0.000	-0.000	
	5 WIND LOA	0.000	6.536	3.105	3.747	-0.000	0.000	
	6 1 DEAD LO	-1.201	2.274	-0.000	-0.105	-0.028	1.378	
	7 1 DEAD LO	1.201	2.274	-0.000	-0.105	0.028	-1.378	
	8 1 DEAD LO	-0.000	-8.167	-4.926	-6.070	0.000	-0.000	
	9 1 DEAD LO	0.000	8.810	3.105	3.642	-0.000	0.000	
	10 1 DEAD L	-1.201	-8.167	-4.926	-6.070	-0.028	1.378	
	11 1 DEAD L	-1.201	8.810	3.105	3.642	-0.028	1.378	
	12 1 DEAD L	1.201	-8.167	-4.926	-6.070	0.028	-1.378	
	13 1 DEAD L	1.201	8.810	3.105	3.642	0.028	-1.378	

Approved ~  
  
 Dr. KHALID MOIN  
 Professor  
 Deptt. of Civil Engineering  
 FGD Engineering & Technology  
 Jamia Millia Islamia  
 New Delhi-110035

## 6. UTILITY RATIO:

## **7. CONCLUSION:**

The structure is found to be safe and the maximum utility ratio observed is 0.896. The IS Code referred during analysis is IS 801- Code of practice for use of cold-formed light gauge steel structural members in general building construction, IS 875- Part 3 : Wind Loads and IS 875-Part 1: Dead Loads.

Approved :  
Head Loads.  
**Dr. KHALID MOIN**  
Professor  
Dept. of Civil Engineering  
FST Engineering & Technology  
Jemia Millat Islamic  
University, Quetta 110325

## 8. STAAD INPUT DATA:

```
1 STAAD SPACE DIAF SHORT OF PUPP-2BMODULE.DAF
2 START FOR INFORMATION
3 ENGINEER DATE 03-Jun-20
4 END FOR INFORMATION
5 INPUT WIDTH 79
6 UNIT METER KN
7 POINT COORDINATES
8 1 0 -672.828 -2852.78; 2 0 -672.86 -2852.78; 3 0 -672.949 -2852.79;
9 5 0.9 -672.758 -2852.84; 6 0.9 -672.984 -2852.74; 7 0.9 -672.893 -2852.78;
10 8 0.9 -672.902 -2852.87; 9 0.9 -672.797 -2852.81; 10 0.9 -672.88 -2852.89;
11 11 0.9 -672.12 -2851.84; 12 0.9 -672.018 -2852.6; 13 0.9 -672.837 -2853.01;
14 14 0.9 -672.319 -2853.18; 15 -0.9 -672.726 -2852.84; 16 -0.9 -672.984 -2853.74;
17 -0.9 -672.893 -2853.78; 18 -0.9 -672.981 -2853.87;
19 -0.9 -672.797 -2852.83; 20 -0.9 -672.88 -2852.89; 21 -0.9 -672.11 -2851.84;
22 -0.9 -672.008 -2852.4; 23 -0.9 -672.837 -2853.03; 24 -0.9 -672.319 -2853.58;
25 25 0.9 -672.028 -2852.78; 26 -0.9 -672.838 -2852.78; 27 0 -672.391 -2852.29;
28 0.7 -672.828 -2852.78; 29 -0.7 -672.828 -2852.78; 30 0.9 -672.122 -2854;
31 32 -0.9 -672.122 -2854; 33 -0.8 -672.122 -2854; 34 0.8 -672.488 -2853.22;
35 35 -0.9 -672.488 -2853.22; 36 -1.5 -672.488 -2853.22; 37 0.9 -672.983 -2852.2;
38 44 -0.3 -672.983 -2853.2; 45 -1.5 -672.983 -2852.2; 46 0.9 -672.327 -2851.42;
39 47 0.9 -672.327 -2851.42; 51 -1.5 -672.327 -2851.42; 54 0.9 -672.088 -2854.97;
40 55 0.9 -672.261 -2852.35; 56 -0.9 -672.088 -2854.87; 57 -0.9 -672.261 -2851.35;
41 58 1.5 -672.122 -2854; 60 1.5 -672.488 -2853.22; 60 1.5 -672.981 -2852.2;
42 61 1.5 -672.327 -2851.42;
43 MEMBER SECTION C93
44 1 2 2; 2 5 10; 4 7 9; 5 9 23; 6 11 13; 7 12 6; 8 13 14; 9 13 20; 10 17 19;
45 11 19 26; 12 21 22; 13 22 19; 14 13 14; 15 25 28; 16 1 29; 17 1 27; 18 2 27;
46 19 2 28; 20 1 29; 21 28 32; 22 23 33; 23 28 38; 27 28 39; 31 42 44; 33 44 45;
47 26 48; 27 28 32; 41 54 58; 42 58 16; 43 28 36; 44 28 7; 45 7 5; 46 3 42;
48 47 42 11; 48 11 48; 49 48 59; 50 56 32; 51 32 24; 52 24 38; 53 28 17; 54 17 15;
49 55 19 44; 56 44 21; 57 21 59; 58 50 37; 63 2 5; 64 28 1; 65 29 26; 66 28 39;
50 67 28 36; 68 68 42; 69 61 48; 70 18 61; 71 9 3; 72 25 38; 73 9 13; 74 6 8;
51 78 26 16; 78 19 18; 77 25 28; 78 18 13; 79 16 12;
52 DEFINE MATERIAL START
53 ISOTROPIC STEEL
54 E 2.05e+003
55 POISSON 0.2
56 DENSITY 78.0129
57 ALPHA 1.2e-003
58 DAMP 0.03
59 TYPE STEEL
60 STRENGTH FY 235200 PU 487888 RV 1.0 ET 1.2
61 END DEFINE MATERIAL
62 MEMBER PROPERTY INDIAN
```

Approved :-

  
Dr. KHALID WAJID  
Professor  
Dept. of Civil Engineering  
PG Engineering & Technology  
Jamia Millia Islamia  
New Delhi-110025

```

44 MEMBER PROPERTY INDIAN
45 1 83 TABLE ST PIP1278H
46 MEMBER PROPERTY COLOFORMED INDIAN
47 43 TO 58 TABLE ST 38PC550H2
48 21 22 26 27 31 32 36 37 66 TO 69 TABLE ST 38C548X1.8
49 MEMBER PROPERTY TATASTRUCTURA
50 21 TABLE ST 38C48X1.8HS
51 18 TABLE ST 48VABE2.4SHS
52 MEMBER PROPERTY COLOFORMED INDIAN
53 6 TO 8 12 TO 14 73 74 76 79 TABLE ST 78C32EX1.8
54 MEMBER PROPERTY INDIAN
55 19 20 TABLE ST PIP1278L
56 8 TO 5 9 TO 11 78 TO 72 75 TO 77 TABLE ST 18A50X30HS
57 15 16 54 55 TABLE ST 78B50X1.8
58 CONSTANTS
59 BETA 100 HHS 41 72 98
60 BETA 335 HHS 21 22 24 27 31 33 36 37 66 TO 69
61 MATERIAL STEEL ALL
62 MEMBER OFFSET
63 21 22 26 27 31 32 36 37 66 TO 69 START 0 0.125 0
64 21 22 26 27 31 32 36 37 66 TO 69 END 0 0.125 0
65 SUPPORTS
66 3 FIXED
67 LOAD 1 LOADTYPE Dead TITLE DEAD LOAD STRUCTURE
68 SELFWEIGHT Y +1
69 MEMBER LOAD
70 21 22 26 27 31 32 36 37 66 TO 69 UNI Y -0.11
71 LOAD 2 LOADTYPE Wind TITLE WIND LOAD X+
72 MEMBER LOAD
73 1 63 UNI GR 0.0323
74 3 4 6 TO 18 12 TO 14 41 TO 58 79 71 73 TO 78 76 75 UNI GR 0.1000
75 LOAD 3 LOADTYPE Wind TITLE WIND LOAD X-
76 MEMBER LOAD
77 1 61 UNI GR -0.0323
78 3 4 6 TO 18 12 TO 14 41 TO 58 79 71 73 TO 78 76 75 UNI GR -0.1000
79 LOAD 4 LOADTYPE Wind TITLE WIND LOAD Z+ UPWARD
80 MEMBER LOAD
81 1 63 UNI GR 0.0323
82 19 20 UNI GR 0.0000
83 21 22 26 27 31 32 36 37 66 TO 69 UNI Y 0.00
84 LOAD 5 LOADTYPE Wind TITLE WIND LOAD Z- DOWNWARD
85 MEMBER LOAD
86 1 63 UNI GR -0.0323
87 19 20 UNI GR -0.0000
88 21 22 26 27 31 32 36 37 66 TO 69 UNI Y -0.0001
89 LOAD COMB 6 1 DEAD LOAD + 2 WIND LOAD X+
90 1 1.0 ± 1.0

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*Approved :-*

*W*  
 Dr. KHALID MOHIN  
 Professor  
 Deptt. of Civil Engineering  
 R/D Engineering & Technology  
 Jamia Millia Islamia  
 New Delhi-110025

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90 1 1.8 2 1.8
91 LOAD CORN 7 1 DEAD LOAD + 1 WIND LOAD X-
92 1 1.8 3 1.8
93 LOAD CORN 3 1 DEAD LOAD + 1 WIND LOAD Z+ UPWARD
94 1 1.8 4 1.8
95 LOAD CORN 9 1 DEAD LOAD + 1 WIND LOAD Z- DOWNWARD
96 1 1.8 5 1.8
97 LOAD CORN 18 1 DEAD LOAD + 1 WIND LOAD X+ + 1 WIND LOAD Z+ UPWARD
98 3 1.8 2 1.8 4 1.8
99 LOAD CORN 11 1 DEAD LOAD + 3 WIND LOAD X+ + 3 WIND LOAD Z- DOWNWARD
100 1 1.8 2 1.8 5 1.8
101 LOAD CORN 12 1 DEAD LOAD + 2 WIND LOAD X- + 1 WIND LOAD Z+ UPWARD
102 5 1.8 4 1.8 1 1.8
103 LOAD CORN 13 1 DEAD LOAD + 1 WIND LOAD X- + 1 WIND LOAD Z- DOWNWARD
104 1 1.8 3 1.8 5 1.8
105 PERFORM ANALYSIS PRINT ALL
106 PARAMETER 1
107 CODE ISB801
108 PVLD 230000 MEMB 1 3 TO 9 9 TO 11 15 TO 28 45 54 63 TO 85 28 TO 71 78 TO 77
109 RATIO 1 MEMB 1 3 TO 9 9 TO 11 15 TO 28 45 54 63 TO 85 28 TO 72 79 TO 77
110 TRACK 1 MEMB 1 3 TO 9 9 TO 11 15 TO 28 45 54 63 TO 85 28 TO 72 79 TO 77
111 CHECK CODE MEMB 1 3 TO 9 9 TO 11 15 TO 28 45 54 63 TO 85 28 TO 72 79 TO 77
112 PARAMETER 2
113 CODE ISB801
114 PU 450000 MEMB 6 TO 8 12 TO 14 21 22 28 27 31 32 36 37 41 TO 58 68 TO 89 73 -
115 74 78 79
116 PVLD 350000 MEMB 6 TO 8 12 TO 14 21 22 28 27 31 32 36 37 41 TO 58 68 TO 89 -
117 74 78 79
118 RATIO 1 MEMB 6 TO 8 12 TO 14 21 22 28 27 31 32 36 37 41 TO 58 68 TO 89 73 -
119 74 78 79
120 TRACK 1 MEMB 6 TO 8 12 TO 14 21 22 28 27 31 32 36 37 41 TO 58 68 TO 89 73 -
121 74 78 79
122 CHECK CODE MEMB 6 TO 8 12 TO 14 21 22 28 27 31 32 36 37 41 TO 58 68 TO 89 -
123 74 78 79
124 PARAMETER 3
125 CODE ISB801
126 STEEL MEMBERS TAKE OFF ALL
127 FINISH

```

Approved :-

Dr. ISHVALID MOHIN  
 Professor  
 Deptt. of Civil Engineering  
 P.G. Engineering & Technology  
 Janki Mills, Ishanis  
 New Delhi-110025

## 9. STAAD OUTPUT:

STAAD.Pro CODE CHECKING - (IS-800:1984) v1.1

\*\*\*\*\*

ALL UNITS ARE - MM MKN (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
--------	-------	---------------	----------------------	--------------	----------------------

### 3 BY PIP1270M (INDIAN SECTIONS)

PASS	IS-7.1.1(A)	0.613	10
2.42 C	0.00	5.35	1.01

ALLOWABLE STRESSES FOR MEMB	1	UNIT - MPa	
FCZ=163.58	FTZ=165.00	FCY=165.00	FTY=165.00 FA=147.49 FV=100.0

### 3 BY ISA50X50X5 (INDIAN SECTIONS)

PASS	IS-7.1.1(A)	0.276	10
0.37 C	0.11	0.06	0.12

ALLOWABLE STRESSES FOR MEMB	3	UNIT - MPa	
FCZ=144.59	FTZ=165.00	FCY=165.00	FTY=165.00 FA=149.61 FV=100.0

### 4 BY ISA50X50X5 (INDIAN SECTIONS)

PASS	IS-7.1.1(A)	0.147	11
0.77 C	0.05	0.03	0.12

ALLOWABLE STRESSES FOR MEMB	4	UNIT - MPa	
FCZ=144.59	FTZ=165.00	FCY=165.00	FTY=165.00 FA=149.61 FV=100.0

### 5 BY ISA50X50X5 (INDIAN SECTIONS)

PASS	IS-7.1.1(A)	0.784	10
0.44 C	0.05	-0.26	0.08

ALLOWABLE STRESSES FOR MEMB	5	UNIT - MPa	
FCZ=144.82	FTZ=165.00	FCY=165.00	FTY=165.00 FA=149.87 FV=100.0

### 9 BY ISA50X50X5 (INDIAN SECTIONS)

PASS	IS-7.1.1(A)	0.276	12
0.37 C	-0.11	0.06	0.12

ALLOWABLE STRESSES FOR MEMB	9	UNIT - MPa	
FCZ=144.59	FTZ=165.00	FCY=165.00	FTY=165.00 FA=149.61 FV=100.0

DXF IMPORT OF PUMP-10MODULE.DXF

-- PAGE-300 18-Nov-2014

Dr. KHANID MOGI  
CIVIL & CHILL ENGINEERING  
TDC Engineering & Technology  
Plot No. 10, Sector 10, Noida  
Uttar Pradesh-201301  
India  
www.Delhi-110025

ALL UNITS ARE - IN METRE (UNLESS OTHERWISE Noted)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ M2	LOADING/ LOCATION
10 ST ISAB50X50X5 (INDIAN SECTIONS)					
	PASS	IS-7.1.1(A)	0.147	13	
	0.77 C	-0.05	0.03	0.12	
ALLOWABLE STRESSES FOR MEMB 10 UNIT - MPa					
	FCS=144.59 FTE=165.00 FCY=165.00 FTY=165.00 FA=149.61 FV=100.0				
11 ST ISAB50X50X5 (INDIAN SECTIONS)					
	PASS	IS-7.1.1(A)	0.784	12	
	0.44 C	-0.05	-0.26	0.08	
ALLOWABLE STRESSES FOR MEMB 11 UNIT - MPa					
	FCS=144.82 FTE=165.00 FCY=165.00 FTY=165.00 FA=149.87 FV=100.0				
15 ST TUB63633.6 (INDIAN SECTIONS)					
	PASS	IS-7.1.2	0.567	4	
	0.35 T	-0.32	-1.08	0.20	
ALLOWABLE STRESSES FOR MEMB 15 UNIT - MPa					
	FCS=164.89 FTE=165.00 FCY=165.00 FTY=165.00 FA=150.00 FV=100.0				
16 ST TUB63633.6 (INDIAN SECTIONS)					
	PASS	IS-7.1.1(A)	0.994	10	
	4.46 C	-2.00	0.37	0.00	
ALLOWABLE STRESSES FOR MEMB 16 UNIT - MPa					
	FCS=162.57 FTE=165.00 FCY=165.00 FTY=165.00 FA=145.42 FV=100.0				
17 ST 80X40X3.2RHS (TATA STRUCTURAL SECTIONS)					
	PASS	IS-7.1.1(A)	0.270	10	
	0.51 C	0.00	-0.56	0.00	
ALLOWABLE STRESSES FOR MEMB 17 UNIT - MPa					
	FCS=156.15 FTE=165.00 FCY=165.00 FTY=165.00 FA=133.65 FV=100.0				
DXF IMPORT OF PUMP-1MODULE.DXF					
-- PAGE NO. 19					

Approved :-

Dr. KHALID MOIN  
Professor  
Dept. of Civil Engineering  
IITG-Engineering & Technology  
Jania Millia Islamia  
New Delhi-110025

ALL UNITS ARE - IN METRE (UNLESS OTHERWISE Noted)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
--------	-------	---------------	----------------------	--------------	----------------------

18 ST 40X60X2.6SHS (INDIAN STRUCTURAL SECTIONS)

PASS	IS-7.1.1(A)	0.436	10
7.29 C	-0.02	0.26	0.00

| ALLOWABLE STRESSES FOR MEMB 18 UNIT - MPA  
| FCB=157.94 FTZ=165.00 FCY=165.00 FTY=165.00 FA=134.99 FW=100.0 |

19 ST PIP337L (INDIAN SECTIONS)

PASS	IS-7.1.1(A)	0.533	11
7.29 C	0.00	0.04	0.00

| ALLOWABLE STRESSES FOR MEMB 19 UNIT - MPA  
| FCB=134.71 FTZ=165.00 FCY=165.00 FTY=165.00 FA= 70.26 FW=100.0 |

20 ST PIP337L (INDIAN SECTIONS)

PASS	IS-7.1.1(A)	0.533	13
7.29 C	0.00	0.04	0.00

| ALLOWABLE STRESSES FOR MEMB 20 UNIT - MPA  
| FCB=134.71 FTZ=165.00 FCY=165.00 FTY=165.00 FA= 70.26 FW=100.0 |

45 ST 100CS60X2 (IS COLDFORMED SECTIONS)

PASS	IS-7.1.1(A)	0.423	10
7.69 C	0.18	0.33	0.15

| ALLOWABLE STRESSES FOR MEMB 45 UNIT - MPA  
| FCB=164.92 FTZ=165.00 FCY=165.00 FTY=165.00 FA=149.91 FW=100.0 |

54 ST 100CS60X2 (IS COLDFORMED SECTIONS)

PASS	IS-7.1.1(A)	0.423	12
7.69 C	-0.18	0.33	0.15

| ALLOWABLE STRESSES FOR MEMB 54 UNIT - MPA  
| FCB=164.92 FTZ=165.00 FCY=165.00 FTY=165.00 FA=149.91 FW=100.0 |

DXF IMPORT OF PUMP-10MODULE.DXF

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ALL UNITS ARE - IN METRE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
--------	-------	---------------	----------------------	--------------	----------------------

  
 Dr. KHALID NOOR  
 Professor  
 Deptt. of Civil Engineering  
 P/O Engineering & Technology  
 Jamia Millia Islamia  
 New Delhi-110025

63 ST PIP127OM (INDIAN SECTIONS)

		PASS	IS-7.1.2	0.726	10	
-----		8.17 T	0.00	6.22	0.10	
-----		ALLOWABLE STRESSES FOR MEMB 63 UNIT - MPa				
-----		FCB=164.96 FTE=165.00 FCY=165.00 FTY=165.00 FA=150.00 FV=100.0				
-----		(INDIAN SECTIONS)				
64 ST		TUB63633.6	PASS	IS-7.1.1(A)	0.994	12
			4.46 C	-2.00	0.37	0.70
-----		ALLOWABLE STRESSES FOR MEMB 64 UNIT - MPa				
-----		FCB=162.57 FTE=165.00 FCY=165.00 FTY=165.00 FA=145.42 FV=100.0				
-----		(INDIAN SECTIONS)				
65 ST		TUB63633.6	PASS	IS-7.1.2	0.567	4
			0.35 T	-0.32	-1.08	0.00
-----		ALLOWABLE STRESSES FOR MEMB 65 UNIT - MPa				
-----		FCB=164.89 FTE=165.00 FCY=165.00 FTY=165.00 FA=150.00 FV=100.0				
-----		(INDIAN SECTIONS)				
70 ST		IS8A50X50X5	PASS	IS-7.1.1(A)	0.262	4
			1.66 C	-0.00	0.10	0.12
-----		ALLOWABLE STRESSES FOR MEMB 70 UNIT - MPa				
-----		FCB=144.59 FTE=165.00 FCY=165.00 FTY=165.00 FA=149.61 FV=100.0				
-----		(INDIAN SECTIONS)				
71 ST		IS8A50X50X5	PASS	IS-7.1.1(A)	0.234	4
			2.73 C	0.00	-0.07	0.11
-----		ALLOWABLE STRESSES FOR MEMB 71 UNIT - MPa				
-----		FCB=144.62 FTE=165.00 FCY=165.00 FTY=165.00 FA=149.65 FV=100.0				
-----		DXF IMPORT OF PUMP-10MODULE.DXF				
				-- PAGE NO. 21		

ALL UNITS ARE - MM MEASURED (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COMB/ HY	RATIO/ ME	LOADING/ LOCATION
--------	-------	---------------	----------------------	--------------	----------------------

72 ST		IS8A50X50X5	(INDIAN SECTIONS)		
-----		PASS	IS-7.1.2	0.271	8
-----		0.05 T	-0.00	0.10	0.00
-----		ALLOWABLE STRESSES FOR MEMB 72 UNIT - MPa			
-----		FCB=144.82 FTE=165.00 FCY=165.00 FTY=165.00 FA=150.00 FV=100.0			

*Approved :-*

*Khalid Mohi*

Projector  
Dept. of Civil Engineering  
P/O Engineering & Technology  
Aero India Mianwala  
New Delhi-110034

75 ST ISAB50X50X5 (INDIAN SECTIONS)

PASS	IS-7.1.1(A)	0.262	4
1.66 C	0.00	0.10	0.12

ALLOWABLE STRESSES FOR MEMB 75 UNIT - MPa  
FCZ=144.59 FTD=165.00 FCY=165.00 FTY=165.00 FA=149.61 FV=100.0

76 ST ISAB50X50X5 (INDIAN SECTIONS)

PASS	IS-7.1.1(A)	0.234	4
2.73 C	-0.00	-0.07	0.11

ALLOWABLE STRESSES FOR MEMB 76 UNIT - MPa  
FCZ=144.62 FTD=165.00 FCY=165.00 FTY=165.00 FA=149.55 FV=100.0

77 ST ISAB50X50X5 (INDIAN SECTIONS)

PASS	IS-7.1.2	0.271	8
0.05 T	0.00	0.10	0.08

ALLOWABLE STRESSES FOR MEMB 77 UNIT - MPa  
FCZ=144.82 FTD=165.00 FCY=165.00 FTY=165.00 FA=150.00 FV=100.0

\*\*\*\*\* END OF TABULATED RESULT OF DESIGN \*\*\*\*\*

112. PARAMETER 2

113. CODE IS801

DXF IMPORT OF PUMP-10MODULE.DXF

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114. FU 450000 MEMB 6 TO 8 12 TO 14 21 22 26 27 31 32 36 37 41 TO 58 66 TO 69 73 -  
115. 74 78 79

116. FYLD 350000 MEMB 6 TO 8 12 TO 14 21 22 26 27 31 32 36 37 41 TO 58 66 TO 69 -  
117. 73 74 78 79

118. RATIO 1 MEMB 6 TO 8 12 TO 14 21 22 26 27 31 32 36 37 41 TO 58 66 TO 69 73 -  
119. 74 78 79

120. TRACK 1 MEMB 6 TO 8 12 TO 14 21 22 26 27 31 32 36 37 41 TO 58 66 TO 69 73 -  
121. 74 78 79

122. CHECK CODE MEMB 6 TO 8 12 TO 14 21 22 26 27 31 32 36 37 41 TO 58 66 TO 69 -  
123. 73 74 78 79

DXF IMPORT OF PUMP-10MODULE.DXF

-- PAGE NO. 23

STAAD.Pro CODE CHECKING - (IS-800:1984) v1.1

\*\*\*\*\*

UNITS : MM, KN, KNM, MPa

MEMBERS 6 SECTION: 70CB25H1.6 LEN: 766.83 GOV. LOC: 0.00 Dr. KHALID MOIN  
Dept. of Civil Engineering & Technology  
FJCI Engineering & Technology  
Jamia Millia Islamia  
New Delhi 110025

```
| STATUS: PASS RATIO = 0.315 GOV.MODE: Bend + Compress GOV.LOAD: 13 |
| |
| RESISTANCES: AX.TENS: 0.00 COMPRESS: 31.32 |
| BEND. Z: 0.84 BEND. Y: 0.00 SHEAR Z: 7.62 SHEAR Y: 13.89 |
|-----|
```

```
|-----|
| UNITS : MM, KN, KNM, MPA |
| |
| MEMBERS 7 SECTION: 70CS25X1.6 LEN: 154.02 GOV.LOC: 154.02 |
| STATUS: PASS RATIO = 0.404 GOV.MODE: Bend + Compress GOV.LOAD: 13 |
| |
| RESISTANCES: AX.TENS: 0.00 COMPRESS: 35.22 |
| BEND. Z: 0.87 BEND. Y: 0.00 SHEAR Z: 7.62 SHEAR Y: 13.89 |
|-----|
```

```
|-----|
| UNITS : MM, KN, KNM, MPA |
| |
| MEMBERS 8 SECTION: 70CS25X1.6 LEN: 770.31 GOV.LOC: 770.31 |
| STATUS: PASS RATIO = 0.351 GOV.MODE: Bend + Compress GOV.LOAD: 13 |
| |
| RESISTANCES: AX.TENS: 0.00 COMPRESS: 31.29 |
| BEND. Z: 0.69 BEND. Y: 0.00 SHEAR Z: 7.62 SHEAR Y: 13.89 |
|-----|
```

```
|-----|
| UNITS : MM, KN, KNM, MPA |
| |
| MEMBERS 12 SECTION: 70CS25X1.6 LEN: 766.83 GOV.LOC: 0.00 |
| STATUS: PASS RATIO = 0.315 GOV.MODE: Bend + Compress GOV.LOAD: 11 |
| |
| RESISTANCES: AX.TENS: 0.00 COMPRESS: 31.32 |
| BEND. Z: 0.70 BEND. Y: 0.00 SHEAR Z: 7.62 SHEAR Y: 13.89 |
|-----|
```

```
|-----|
| DKP IMPORT OF FUMP-10MODULE.DKF -- PAGE NO. 24 |
|-----|
```

```
|-----|
| UNITS : MM, KN, KNM, MPA |
| |
| MEMBERS 13 SECTION: 70CS25X1.6 LEN: 154.02 GOV.LOC: 154.02 |
| STATUS: PASS RATIO = 0.404 GOV.MODE: Bend + Compress GOV.LOAD: 11 |
| |
| RESISTANCES: AX.TENS: 0.00 COMPRESS: 35.22 |
| BEND. Z: 0.87 BEND. Y: 0.00 SHEAR Z: 7.62 SHEAR Y: 13.89 |
|-----|
```

Dr. IJAHIA MOIN  
Professor  
Dept. of Civil Engineering  
FUD Engineering & Technology  
Jinnah Islamic University  
New Faisalabad

UNITS : MN, KN, KNM, MPa  
 MEMBERS 14 SECTION: 70CH25X1.6 LEN: 770.31 GOV.LOC: 770.31  
 STATUS: PASS RATIO = 0.351 GOV.MODE: Bend + Compress GOV.LOAD: 11  
 RESISTANCES: AX.TENS: 0.00 COMPRESS: 31.29  
 BEND. Z: 0.84 BEND. Y: 0.00 SHEAR Z: 7.62 SHEAR Y: 13.89

UNITS : MN, KN, KNM, MPa  
 MEMBERS 21 SECTION: 80CS40X1.6 LEN: 1800.00 GOV.LOC: 0.00  
 STATUS: PASS RATIO = 0.244 GOV.MODE: Bending Z GOV.LOAD: 8  
 RESISTANCES: AX.TENS: 56.48 COMPRESS: 0.00  
 BEND. Z: 0.63 BEND. Y: 0.00 SHEAR Z: 14.34 SHEAR Y: 16.13

UNITS : MN, KN, KNM, MPa  
 MEMBERS 22 SECTION: 80CS40X1.6 LEN: 600.00 GOV.LOC: 0.00  
 STATUS: PASS RATIO = 0.125 GOV.MODE: Bend Z + Shear GOV.LOAD: 4  
 RESISTANCES: AX.TENS: 0.00 COMPRESS: 0.00  
 BEND. Z: 1.44 BEND. Y: 0.00 SHEAR Z: 14.34 SHEAR Y: 16.13

UNITS : MN, KN, KNM, MPa  
 MEMBERS 26 SECTION: 80CS40X1.6 LEN: 1800.00 GOV.LOC: 900.00  
 STATUS: PASS RATIO = 0.343 GOV.MODE: Bending Z GOV.LOAD: 4  
 RESISTANCES: AX.TENS: 0.00 COMPRESS: 26.00  
 BEND. Z: 0.63 BEND. Y: 0.00 SHEAR Z: 0.00 SHEAR Y: 0.00  
 DXF IMPORT OF PUMP-10MOCULE.DXF -- PAGE NO. 25

UNITS : MN, KN, KNM, MPa  
 MEMBERS 27 SECTION: 80CS40X1.6 LEN: 600.00 GOV.LOC: 0.00  
 STATUS: PASS RATIO = 0.125 GOV.MODE: Bend Z + Shear GOV.LOAD: 4  
 RESISTANCES: AX.TENS: 0.00 COMPRESS: 0.00

BEND. Z:	1.44	BEND. Y:	0.00	SHEAR Z:	14.34	SHEAR Y:	16.13
----------	------	----------	------	----------	-------	----------	-------

UNITS : MM, KN, KNM, MPA							
MEMBER#	31	SECTION: 80CH40X1.6	LEN:	1800.00	GOV.LOC:	1200.00	
STATUS:	PASS	RATIO = 0.295	GOV.MODE:	Bending Z	GOV.LOAD:	10	
RESISTANCES:	AX.TENS:	0.00		COMPRESS:	26.00		
BEND. Z:	0.63	BEND. Y:	0.00	SHEAR Z:	14.34	SHEAR Y:	16.13

UNITS : MM, KN, KNM, MPA							
MEMBER#	32	SECTION: 80CH40X1.6	LEN:	600.00	GOV.LOC:	0.00	
STATUS:	PASS	RATIO = 0.125	GOV.MODE:	Bend Z + Shear	GOV.LOAD:	4	
RESISTANCES:	AX.TENS:	0.00		COMPRESS:	0.00		
BEND. Z:	1.44	BEND. Y:	0.00	SHEAR Z:	14.34	SHEAR Y:	16.13

UNITS : MM, KN, KNM, MPA							
MEMBER#	36	SECTION: 80CH40X1.6	LEN:	1800.00	GOV.LOC:	960.00	
STATUS:	PASS	RATIO = 0.341	GOV.MODE:	Bending Z	GOV.LOAD:	4	
RESISTANCES:	AX.TENS:	56.48		COMPRESS:	0.00		
BEND. Z:	0.63	BEND. Y:	0.00	SHEAR Z:	0.00	SHEAR Y:	0.00

DXF IMPORT OF PUMP-10MODULE.DXF				--- PAGE NO. 26			
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UNITS : MM, KN, KNM, MPA							
MEMBER#	37	SECTION: 80CH40X1.6	LEN:	600.00	GOV.LOC:	0.00	
STATUS:	PASS	RATIO = 0.125	GOV.MODE:	Bend Z + Shear	GOV.LOAD:	4	
RESISTANCES:	AX.TENS:	0.00		COMPRESS:	0.00		
BEND. Z:	1.44	BEND. Y:	0.00	SHEAR Z:	14.34	SHEAR Y:	16.13

Approved :-  
Dr. V. U. A. M. Mohan  
Professional Engineer  
Dept. of Civil Engineering  
P.G. Engineering & Technology  
JNTU, Warangal  
Reg. No. 110025

| MEMBER# 41 SECTION: 100CS60X2 LEN: 77.86 GOV.LOC: 77.86 |  
 | STATUS: PASS RATIO = 0.000 GOV.MODE: Bend Y + Shear GOV.LOAD: 3 |  
 |  
 | RESISTANCES: AX.TENS: 0.00 COMPRESS: 0.00 |  
 | BEND. Z: 0.00 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20 |

-----
UNITS : MM, KN, KNM, MPa
-----
MEMBER# 42 SECTION: 100CS60X2 LEN: 463.81 GOV.LOC: 463.81
STATUS: PASS RATIO = 0.231 GOV.MODE: Bend Z + Shear GOV.LOAD: 4
RESISTANCES: AX.TENS: 98.76 COMPRESS: 0.00
BEND. Z: 3.28 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20

-----
UNITS : MM, KN, KNM, MPa
-----
MEMBER# 43 SECTION: 100CS60X2 LEN: 396.81 GOV.LOC: 0.00
STATUS: PASS RATIO = 0.408 GOV.MODE: Bend + Compress GOV.LOAD: 4
RESISTANCES: AX.TENS: 0.00 COMPRESS: 35.97
BEND. Z: 3.28 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20

-----
UNITS : MM, KN, KNM, MPa
-----
MEMBER# 44 SECTION: 100CS60X2 LEN: 486.39 GOV.LOC: 486.39
STATUS: PASS RATIO = 0.408 GOV.MODE: Bend + Compress GOV.LOAD: 4
RESISTANCES: AX.TENS: 0.00 COMPRESS: 35.91
BEND. Z: 3.28 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20

DXF IMPORT OF PUMP-1MODULE.DXF -- PAGE NO. 27

-----
UNITS : MM, KN, KNM, MPa
-----
MEMBER# 45 SECTION: 100CS60X2 LEN: 153.43 GOV.LOC: 0.00
STATUS: PASS RATIO = 0.432 GOV.MODE: Bend + Compress GOV.LOAD: 4
RESISTANCES: AX.TENS: 0.00 COMPRESS: 36.06
BEND. Z: 3.28 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20

APPROVED  
 Dr. KHALID MOIN  
 Professor  
 Deptt. of Civil Engineering  
 IIT Roorkee  
 Jharkhand  
 India  
 Pin-722000  
 e-mail: moin@iitroorkee.ernet.in

UNITS : MM, MN, KNM, MPa  
 |  
 | MEMBER# 46 SECTION: 100CS60X2 LEN: 486.39 GOV.LOC: 0.00  
 | STATUS: PASS RATIO = 0.360 GOV.MODE: Bend + Compress GOV.LOAD: 4  
 |  
 | RESISTANCES: AX.TENS: 0.00 COMPRESS: 35.91  
 | BEND. Z: 3.28 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20

UNITS : MM, MN, KNM, MPa  
 |  
 | MEMBER# 47 SECTION: 100CS60X2 LEN: 396.81 GOV.LOC: 396.81  
 | STATUS: PASS RATIO = 0.357 GOV.MODE: Bend + Compress GOV.LOAD: 4  
 |  
 | RESISTANCES: AX.TENS: 0.00 COMPRESS: 35.87  
 | BEND. Z: 3.28 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20

UNITS : MM, MN, KNM, MPa  
 |  
 | MEMBER# 48 SECTION: 100CS60X2 LEN: 463.81 GOV.LOC: 0.00  
 | STATUS: PASS RATIO = 0.193 GOV.MODE: Bend Z + Shear GOV.LOAD: 4  
 |  
 | RESISTANCES: AX.TENS: 0.00 COMPRESS: 35.93  
 | BEND. Z: 3.28 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20

DXF IMPORT OF PUMP-1MODULE.DXF -- PAGE NO. 28  
 UNITS : MM, MN, KNM, MPa  
 |  
 | MEMBER# 49 SECTION: 100CS60X2 LEN: 77.86 GOV.LOC: 0.00  
 | STATUS: PASS RATIO = 0.000 GOV.MODE: Bend Y + Shear GOV.LOAD: 3  
 |  
 | RESISTANCES: AX.TENS: 0.00 COMPRESS: 0.00  
 | BEND. Z: 0.00 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20

UNITS : MM, MN, KNM, MPa  
 |  
 | MEMBER# 50 SECTION: 100CS60X2 LEN: 77.86 GOV.LOC: 77.86  
 | STATUS: PASS RATIO = 0.000 GOV.MODE: Bend Y + Shear GOV.LOAD: 3

DR. KHALID MOHIB  
 Professor  
 Dept. of Civil Engineering  
 Engg. & Technology  
 Jamia Millia Islamia  
 New Delhi-110025

RESISTANCES:	AX.TENS:	0.00	COMPRESS:	0.00			
BEND. Z:	0.00	BEND. Y:	0.00	SHEAR Z:	28.00	SHEAR Y:	25.20

UNITS : MM, KN, KNM, MPA							
MEMBER#	51	SECTION: 100CS60X2	LEN:	463.81	GOV.LOC:	463.81	
STATUS:	PASS	RATIO = 0.231	GOV.MODE:	Bend Z + Shear	GOV.LOAD:	6	
RESISTANCES:		AX.TENS: 98.76		COMPRESS: 0.00			
BEND. Z:	3.28	BEND. Y:	0.00	SHEAR Z:	28.00	SHEAR Y:	25.20

UNITS : MM, KN, KNM, MPA							
MEMBER#	52	SECTION: 100CS60X2	LEN:	396.81	GOV.LOC:	0.00	
STATUS:	PASS	RATIO = 0.408	GOV.MODE:	Bend + Compress	GOV.LOAD:	4	
RESISTANCES:		AX.TENS: 0.00		COMPRESS: 35.97			
BEND. Z:	3.28	BEND. Y:	0.00	SHEAR Z:	28.00	SHEAR Y:	25.20

UNITS : MM, KN, KNM, MPA							
MEMBER#	53	SECTION: 100CS60X2	LEN:	486.39	GOV.LOC:	486.39	
STATUS:	PASS	RATIO = 0.408	GOV.MODE:	Bend + Compress	GOV.LOAD:	4	
RESISTANCES:		AX.TENS: 0.00		COMPRESS: 35.91			
BEND. Z:	3.28	BEND. Y:	0.00	SHEAR Z:	28.00	SHEAR Y:	25.20

DXF IMPORT OF PUMP-1MODULE.DXF  
-- PAGE NO. 29

UNITS : MM, KN, KNM, MPA							
MEMBER#	54	SECTION: 100CS60X2	LEN:	153.43	GOV.LOC:	0.00	
STATUS:	PASS	RATIO = 0.412	GOV.MODE:	Bend + Compress	GOV.LOAD:	4	
RESISTANCES:		AX.TENS: 0.00		COMPRESS: 36.06			
BEND. Z:	3.28	BEND. Y:	0.00	SHEAR Z:	28.00	SHEAR Y:	25.20

UNITS : MM, KN, KNM, MPA							
--------------------------	--	--	--	--	--	--	--

| MEMBER# 55 SECTION: 100CS60X2 LEN: 496.39 GOV.LOC.: 0.00  
 | STATUS: PASS RATIO = 0.360 GOV.MODE: Bend + Compress GOV.LOAD: 4  
 |  
 | RESISTANCES: AX.TENS: 0.00 COMPRESS: 35.91  
 | BEND. Z: 3.28 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20  
 |

|-----  
 | UNITS : MM, KN, KNM, MPa  
 |  
 | MEMBER# 56 SECTION: 100CS60X2 LEN: 396.81 GOV.LOC: 396.81  
 | STATUS: PASS RATIO = 0.357 GOV.MODE: Bend + Compress GOV.LOAD: 4  
 |  
 | RESISTANCES: AX.TENS: 0.00 COMPRESS: 35.97  
 | BEND. Z: 3.28 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20  
 |

|-----  
 | UNITS : MM, KN, KNM, MPa  
 |  
 | MEMBER# 57 SECTION: 100CS60X2 LEN: 463.81 GOV.LOC: 0.00  
 | STATUS: PASS RATIO = 0.193 GOV.MODE: Bend Z + Shear GOV.LOAD: 4  
 |  
 | RESISTANCES: AX.TENS: 0.00 COMPRESS: 35.93  
 | BEND. Z: 3.28 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20  
 |

|-----  
 | DXF IMPORT OF PUMP-10MODULE.DXF -- PAGE NO. 30  
 |  
 | UNITS : MM, KN, KNM, MPa  
 |  
 | MEMBER# 58 SECTION: 100CS60X2 LEN: 77.86 GOV.LOC: 0.00  
 | STATUS: PASS RATIO = 0.000 GOV.MODE: Bend Y + Shear GOV.LOAD: 3  
 |  
 | RESISTANCES: AX.TENS: 0.00 COMPRESS: 0.00  
 | BEND. Z: 0.00 BEND. Y: 0.00 SHEAR Z: 28.00 SHEAR Y: 25.20  
 |

|-----  
 | UNITS : MM, KN, KNM, MPa  
 |  
 | MEMBER# 66 SECTION: 80CS40X1.6 LEN: 600.00 GOV.LOC: 600.00  
 | STATUS: PASS RATIO = 0.125 GOV.MODE: Bend Z + Shear GOV.LOAD: 4  
 |  
 | RESISTANCES: AX.TENS: 0.00 COMPRESS: 0.00  
 | BEND. Z: 1.44 BEND. Y: 0.00 SHEAR Z: 14.34 SHEAR Y: 16.13  
 |

UNITS : MM, KN, KNM, MPA  
 MEMBER# 67 SECTION: 80CH40X1.6 LEN: 600.00 GOV.LOC: 600.00  
 STATUS: PASS RATIO = 0.125 GOV.MODE: Bend Z + Shear GOV.LOAD: 4  
 RESISTANCES: AX.TENS: 0.00 COMPRESS: 0.00  
 BEND. Z: 1.44 BEND. Y: 0.00 SHEAR Z: 14.34 SHEAR Y: 16.13

UNITS : MM, KN, KNM, MPA  
 MEMBER# 68 SECTION: 80CH40X1.6 LEN: 600.00 GOV.LOC: 600.00  
 STATUS: PASS RATIO = 0.125 GOV.MODE: Bend Z + Shear GOV.LOAD: 4  
 RESISTANCES: AX.TENS: 0.00 COMPRESS: 0.00  
 BEND. Z: 1.44 BEND. Y: 0.00 SHEAR Z: 14.34 SHEAR Y: 16.13

UNITS : MM, KN, KNM, MPA  
 MEMBER# 69 SECTION: 80CH40X1.6 LEN: 600.00 GOV.LOC: 600.00  
 STATUS: PASS RATIO = 0.125 GOV.MODE: Bend Z + Shear GOV.LOAD: 4  
 RESISTANCES: AX.TENS: 0.00 COMPRESS: 0.00  
 BEND. Z: 1.44 BEND. Y: 0.00 SHEAR Z: 14.34 SHEAR Y: 16.13  
 DXF IMPORT OF PUMP-10MODULE.DXF  
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UNITS : MM, KN, KNM, MPA  
 MEMBER# 73 SECTION: 70CH25X1.6 LEN: 153.94 GOV.LOC: 0.00  
 STATUS: PASS RATIO = 0.450 GOV.MODE: Bend + Compress GOV.LOAD: 13  
 RESISTANCES: AX.TENS: 0.00 COMPRESS: 35.22  
 BEND. Z: 0.87 BEND. Y: 0.00 SHEAR Z: 7.62 SHEAR Y: 13.89

UNITS : MM, KN, KNM, MPA  
 MEMBER# 74 SECTION: 70CH25X1.6 LEN: 144.44 GOV.LOC: 144.44  
 STATUS: PASS RATIO = 0.403 GOV.MODE: Bend + Compress GOV.LOAD: 13

RESISTANCES:	AX.TENS:	0.00	COMPRESS:	35.24			
BEND. Z:	0.87	BEND. Y:	0.00	SHEAR Z:	7.62	SHEAR Y:	13.89

UNITS : MM, KN, KNM, MPA							
MEMBERS	76	SECTION: 70CS25X1.6	LEN:	153.84	GOV.LOC:	0.00	
STATUS: PASS	RATIO = 0.450	GOV.MODE: Bend + Compress	GOV.LOAD:	11			
RESISTANCES:	AX.TENS:	0.00	COMPRESS:	35.24			
BEND. Z:	0.87	BEND. Y:	0.00	SHEAR Z:	7.62	SHEAR Y:	13.89

UNITS : MM, KN, KNM, MPA							
MEMBER#	79	SECTION: 70CS25X1.6	LEN:	144.44	GOV.LOC:	144.44	
STATUS: PASS	RATIO = 0.403	GOV.MODE: Bend + Compress	GOV.LOAD:	11			
RESISTANCES:	AX.TENS:	0.00	COMPRESS:	35.24			
BEND. Z:	0.87	BEND. Y:	0.00	SHEAR Z:	7.62	SHEAR Y:	13.89

**DXF IMPORT OF PUMP-10MODULE.DXF**

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124. PARAMETER 3

125. CODE IS801

126. STEEL MEMBER TAKE OFF ALL

**DXF IMPORT OF PUMP-10MODULE.DXF**

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**STEEL TAKE-OFF**

PROFILE	LENGTH(METER)	WEIGHT(KN)
ST PIP1270M	1.12	0.158
ST ISB50X50KS	1.22	0.045
ST 70CE25X1.6	3.98	0.063
ST TUB63633.6	1.80	0.114
ST 80X40X3.2RRS	0.77	0.042
ST 40X40X2.6RRS	0.69	0.020
ST PIP337L	2.46	0.048
ST 80CS40X1.6	12.00	0.248
ST 100CS60X2	6.01	0.217
TOTAL =		0.954

Approved :-

Dr. YATIJO MAIN  
Professor  
Dept. of Civil Engineering  
IIT Engineering & Technology  
Janta Bhawan, Iitanna  
New Delhi-110025

MEMBER	PROFILE	LENGTH (INCHES)	WEIGHT (LBF.)
1	ST PIP1270H	1.01	0.143
3	ST ISAS030X05	0.12	0.004
4	ST ISAS050X05	0.12	0.004
5	ST ISAS050X05	0.08	0.003
6	ST 70CS25X1.6	0.77	0.012
7	ST 70CH25X1.6	0.15	0.002
8	ST 70CS25X1.6	0.77	0.012
9	ST ISAS050X50X5	0.12	0.004
10	ST ISAS050X50X5	0.12	0.004
11	ST ISAS050X50X5	0.08	0.003
12	ST 70CS25X1.6	0.77	0.012
13	ST 70CS25X1.6	0.15	0.002
14	ST 70CS25X1.6	0.77	0.012
15	ST TUB63633.6	0.20	0.013
16	ST TUB63633.6	0.70	0.044
17	ST 80X40X3.2R88	0.77	0.042
18	ST 40X40X2.68ms	0.69	0.020
19	ST PIP337L	1.23	0.024
20	ST PIP337L	1.23	0.024
21	ST 80CS40X1.6	1.80	0.037
22	ST 80CS40X1.6	0.60	0.012
26	ST 80CS40X1.6	1.80	0.037
27	ST 80CS40X1.6	0.60	0.012
31	ST 80CS40X1.6	1.80	0.037
32	ST 80CS40X1.6	0.60	0.012
36	ST 80CS40X1.6	1.80	0.037
37	ST 80CS40X1.6	0.60	0.012
41	ST 100CS60X2	0.08	0.003
42	ST 100CS60X2	0.45	0.017
43	ST 100CS60X2	0.60	0.014
44	ST 100CS60X2	0.49	0.018
45	ST 100CS60X2	0.15	0.006

DXF IMPORT OF PUMP-10MODULE.DXF

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46	ST 100CS60X2	0.49	0.018
47	ST 100CS60X2	0.40	0.014
48	ST 100CS60X2	0.46	0.017
49	ST 100CS60X2	0.08	0.003
50	ST 100CS60X2	0.08	0.003
51	ST 100CS60X2	0.46	0.017
52	ST 100CS60X2	0.40	0.014
53	ST 100CS60X2	0.49	0.018
54	ST 100CS60X2	0.15	0.006
55	ST 100CS60X2	0.49	0.018
56	ST 100CS60X2	0.40	0.014
57	ST 100CS60X2	0.46	0.017
58	ST 100CS60X2	0.08	0.003
63	ST PIP1270H	0.10	0.015

Approved :-



Dr. KHALID MOIN  
Professor  
Dept. of Civil Engineering  
IITD Engineering & Techn.  
Jaini Mills Ismailia  
New Delhi-110025

64	ST	TUB63633.6	0.70	0.044
65	ST	TUB63633.6	0.20	0.013
66	ST	80CS40X1.6	0.60	0.012
67	ST	80CS40X1.6	0.60	0.012
68	ST	80CS40X1.6	0.60	0.012
69	ST	80CS40X1.6	0.60	0.012
70	ST	ISA50X50X5	0.12	0.004
71	ST	ISA50X50X5	0.11	0.004
72	ST	ISA50X50X5	0.08	0.003
73	ST	70CS25X1.6	0.15	0.002
74	ST	70CS25X1.6	0.14	0.002
75	ST	ISA50X50X5	0.12	0.004
76	ST	ISA50X50X5	0.11	0.004
77	ST	ISA50X50X5	0.08	0.003
78	ST	70CS25X1.6	0.15	0.002
79	ST	70CS25X1.6	0.14	0.002
<hr/>				
		TOTAL =		0.954

\*\*\*\*\* END OF DATA FROM INTERNAL STORAGE \*\*\*\*\*

127. FINISH

Approved :-

Dr. KHALID MOHIM  
Professor  
Dept. of Civil Engineering  
F/O Engineering & Technology  
Jamia Millia Islamia  
New Delhi-110025

Approved :-

Dr. KHALID MOHIM  
Professor  
Dept. of Civil Engineering  
F/O Engineering & Technology  
Jamia Millia Islamia  
New Delhi-110025

**DESIGN BASIS REPORT**  
**FOR**  
**9 MODULE SOLAR PUMP**  
**MOUNTING STEEL STRUCTURE TABLE**

For  
ETHOS POWER PVT LTD.  
121-123, 1st Floor, NH - 8 DLF Star Tower, Sector  
30, Gurugram, Haryana, India - 122001

Date: 06<sup>th</sup> JUNE 2020

Approved by  
Dr. RAJALI MOHAN  
Professor  
Dept. of Civil Engineering  
PRO Engineering & Technology  
Janak Puri Extension  
New Delhi-110033

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*Approved :-*

*M*  
 Dr. KHALID MOIN  
 Professor  
 Deptt. of Civil Engineering  
 P/O Engineering & Technology  
 Jamia Millia Islamia  
 New Delhi-110025

## **1. GENERAL:**

### **2.1 SCOPE**

This document covers the Analysis & Design of Fixed type Solar PV Module mounting structure at an angle of tilt 25 degree for at 155kmph wind speed.

### **2.2 UNITS OF MEASUREMENTS**

Units of measurements used in analysis shall be of SI Units.

### **2.3 CODES AND REFERENCE**

Following have been referred for analysis and design of the MMS

Project Reference Drawings/Documents		
1	-----	Geo-Technical Investigation
2	-----	Plot Plan
3	-----	Module GA
Indian Standard Codes		
4	IS 456 - 2000	Plain and Reinforced Concrete - Code of Practice
5	IS 875: Part 1 & 2	Code of practice for the design loads for building and structures
6	IS 875: Part 3 - 2015	Code of practice for the design loads for building and structures - Wind Loads
7	IS 800: 2007 (WSD)	Code of practice for general construction in Steel of hot rolled
8	IS 801: 1975	Code of practice for cold formed steel
9	IS 811: 1987	Specifications for cold formed light gauge structural steel sections
10	IS: 2062	Hot rolled medium and high tensile structural steel

## 2.4 MATERIAL PROPERTIES:

Material	Property	Value	Units
Concrete - M25	Density	25	kN/m <sup>3</sup>
	Characteristic Strength	25	N/m <sup>2</sup>
	Modulus of Elasticity	25000	N/m <sup>2</sup>
High Strength Reinforcing Steel	Density	78.5	kN/m <sup>3</sup>
	Characteristic Strength	500	N/m <sup>2</sup>
	Modulus of Elasticity	200000	N/m <sup>2</sup>
Structural Steel	Density	78.5	kN/m <sup>3</sup>
	Characteristic Strength	350000	kN/m <sup>2</sup>
	Modulus of Elasticity	250000	N/m <sup>2</sup>

## 2.5 PLANT INFORMATION:

Location of Site: India  
Basic Wind Speed: 43 m/s (155kmph)

## 2.6 SOFTWARE USED:

Below listed software are used for the structural analysis and design;

S. No.	Software Name	Developed By	Usages
1	STAAD Pro	Research Engineers Inc., Berkely, USA	Structural Analysis And Design
2	In-house developed MS -Excel Programs	.....	Load Calculations

Approved -

Dr. KHALID MOIN  
Professor  
Dept. of Civil Engineering  
FVD Engineering & Technology  
Jauna Mehta Islamic  
New Delhi-110025

## 2. LOADS:

### 2.1 Load Calculation:

Dead Load Calculation:-		UDL LOAD		
Module	=			
Solar module Dimension	=	Length (mm)	Width (mm)	Nos
		1964	986	9
Weight per module	=	22.5	Kg's/module	
Weight of 9 modules	=	202.5	Kg's	
Module Configuration	=	3x2	Portrait	
Including 10% extra Load per for connections	=	202.5	Kg's	
Purlin Length	=	9.02	m	
Total Length of purlin	=	18.04	m	
Udl load at purlin	=	0.110	kN/m	
All dead loads have been applied as udl load by considering the modules are in constant touch with the purlin upper face.				

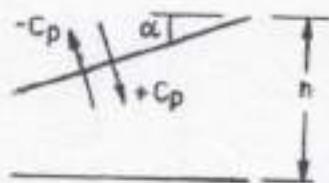
#### 2.1.1 Wind Load:

Wind Pressure Calculation				
Basic Wind Speed	=	155.00	mph	
Basic Wind Speed	=	43	ms/sec	
Design Period	=	25	years	As per Tyndall Specification
Type of NBIS	=	Ground Based		
Terrain Category	=	2		
Class	=	A		
Module Clearance From NGI	=	0.85	m	
		Length	Width	Nos
Solar module Dimension	=	1964	986	9
Roof Co-efficiency, k1	=	0.91	(As per Table-1, IS-875 part-3)	
Terrain Factor, k2	=	1.00	For Terrain Category 2	
Topography factor, k3	=	1.00	(As per Table-2 of IS: 875 Part 3- 1987)	
Importance factor, k4	=	1.00	(Refer IS-875 part-3 clause 6.3.4)	
Design Wind Speed, $V_d = k_1 \cdot k_2 \cdot k_3 \cdot k_4 \cdot V_{ref}$	=	39.18	m/sec	
Design Wind pressure, $P_d = 0.002(V_d)^2$	=	707.56	N/sqm	
As per Latest code IS : 875 (Part-3-2015)				
The design wind pressure $p_d$ can be obtained as as per clause 4.5.2(pd = k1, ka, kr, Pd)				
Where,				
Wind directivity factor, k1	=	0.90	(Refer clause-7.2.1, In IS-875 part-3)	
Area of solar panels	=	17.43		
Area averaging factor, k2	=	0.96	(if this area is == 25 than $k_2 = 0.9$ )	
Combination factor, k3	=	1	(Refer clause-7.3.3.15, In IS-875 part-3)	
Design pressure shall be, $p_d = k_1, ka, kr, P_d$	=	665.17	N/sqm	

Approved :-

Dr. KHALID MOIN  
Professor  
Dept. of Civil Engineering  
F/O Engineering & Technology  
Jinnah Miller Institute  
New Delhi 110029

## Calculating pressure coefficient for mono-slope roof;



### SIGN CONVENTION

+ TOWARDS THE STRUCTURE (SUCTION), - AWAY FROM THE STRUCTURE (UPLIFT)

For calculating the force on the panels from the pressure calculated above, we need pressure coefficient. The current structure falls under mono-slope free roofs. Pressure coefficient for mono-slope roof is found out referring section 7.3.3.2, Table 8, IS: 875, Part 3 - 2015. The pressure coefficients for a mono-slope free roof of 05° tilt angle and solidity ratio of 0.

### WIND LOAD UPWARD/DOWNWARD (+/- Y DIRECTION)

Solar module Dimension	=	Length 1000	Width 900	Nos 9
Tilt angle of MDR for now	=	25	Degrees	
Upward Decreased Wind Force, $F_u = C_p * A * P_d$ Where,	=			
Pressure Coefficient for Upward wind, $C_p$ Surface Area of Structural element, $A = 1000 \times 900 \times 0.05 = 45000$	=	-1.0		(As per IS875 Part-3, Table-8)
Design wind pressure, $P_d$	=	17.43	mm	
Upward Wind Force, $F_u = C_p * A * P_d$	=	103.37	N/mm²	
Pressure coefficient for Downward wind, $C_p$	=	1.00		(As per IS875 Part-3, Table-8)
Downward Wind Force, $F_d = C_p * A * P_d$	=	11.50	N/mm²	
Here, we are considering split wind force of Solar panels shall be bear by Panels				
No. of Panels support	=	2	Nos	
Length of member	=	0.72	m	
Upward Wind force, $F_u = F_u / (2 \times 0.02)$	=	-1.03	N/mm	
Downward Wind force, $F_d = F_d / (2 \times 0.02)$	=	0.643	N/mm	

Where, Negative sign shows Upward direction of Wind

Approved :-

Dr. KHALID MOIN  
Professor  
Dept. of Civil Engineering  
FIO Engineering & Technology  
Jameel Miller Islamic  
New Campus-150025

WIND LOAD SIDEWAYS (X & Z DIRECTIONS)					
MEM ELEMENTS		SECTION DETAILS			
Column	-	Web	Flange	Top	Bottom
Rafter	-	130	130	8	2.4
BRACING	-	47	47	6	3.2
<i>As per table-28, IS-875 part-2</i>					
Design Wind speed, $V_d$	-	29.18	none		
Design Wind pressure, $P_d$	-	0.0317	0.0317		
<i>Secthues</i>					
Column	Angle of Wind	Cb	Wind facing width in mm(D) (Z-direction)	Length of member in mm(l)	Dof. in m/sec. (X-direction)
Rafter	0°	0.31	130.7	1400	3
BRACING	0°	2.01	47	3100	3
<i>Vertices</i>					
Column	Angle of Wind	Cb	Wind facing width in mm(D) (Z-direction)	Length of member in mm(l)	Dof. in m/sec. (Z-direction)
BRACING	0°	0.31	130.7	1400	3
Wind force at Column per meter End width "cm"²	-	0.0330	None		
Wind force at Rafter per meter End width "cm"²	-	0.1370		X-direction	
Wind force at Column per meter End width "cm"²	-	0.0300	None		
Wind force at Rafter per meter End width "cm"²	-	0.0316		Z-direction	

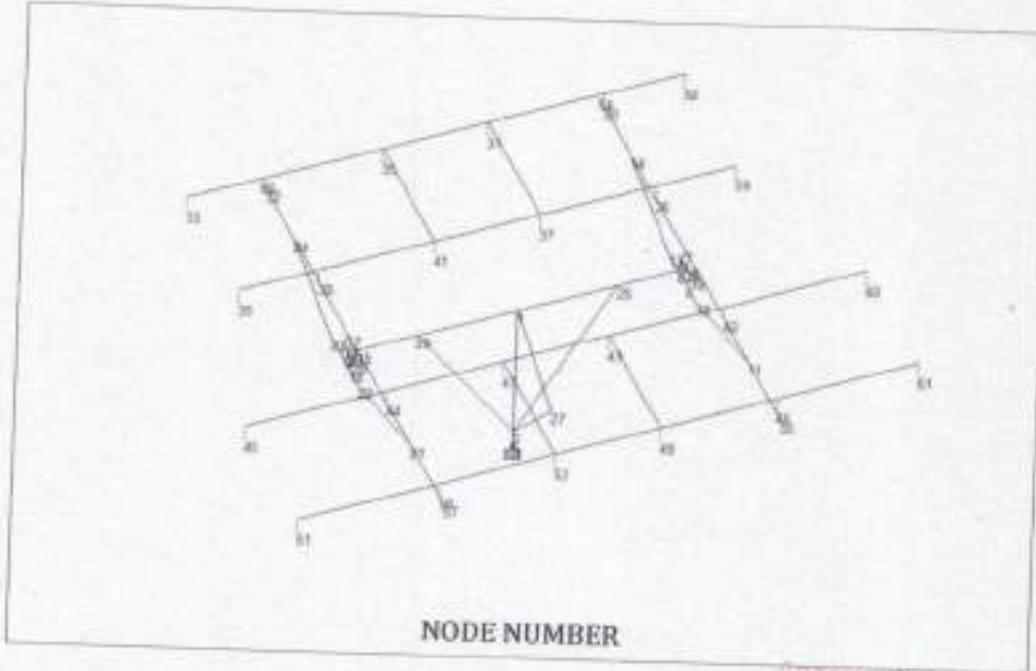
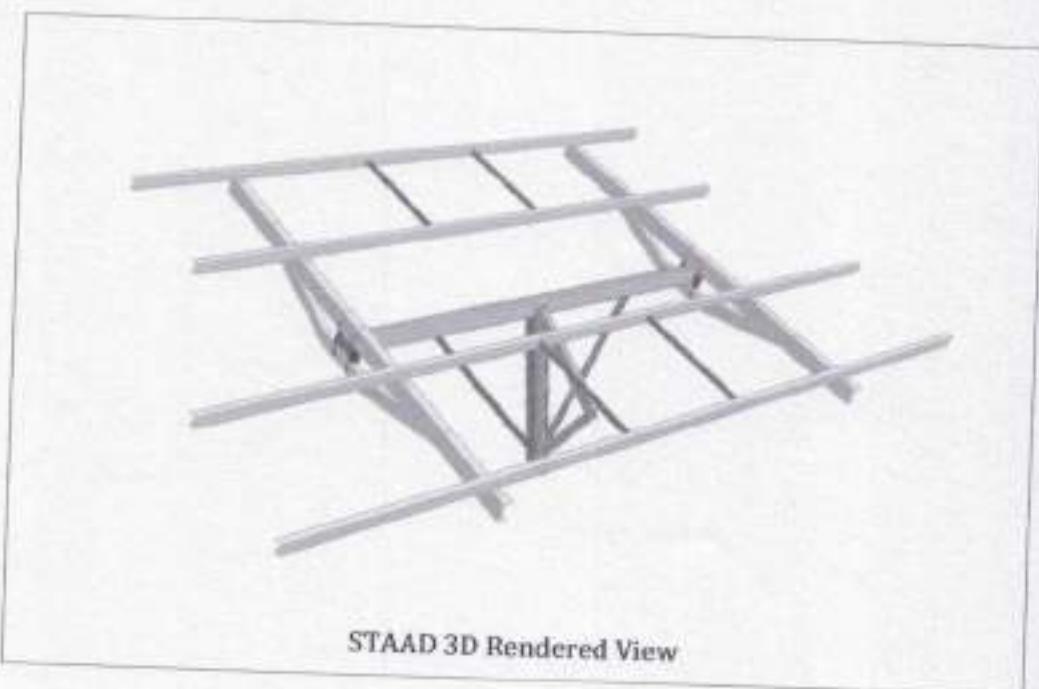
## 2.2 LOAD COMBINATION

LOAD TYPE	L/C	LOAD NAME
Primary	1	DEAD LOAD STRUCTURE
Primary	2	WIND LOAD X+
Primary	3	WIND LOAD X-
Primary	4	WIND LOAD Z+ UPWARD
Primary	5	WIND LOAD Z- DOWNWARD
Combination	6	I DEAD LOAD + I WIND LOAD X+
Combination	7	I DEAD LOAD + I WIND LOAD X-
Combination	8	I DEAD LOAD + I WIND LOAD Z+ UPWARD
Combination	9	I DEAD LOAD + I WIND LOAD Z- DOWNWARD
Combination	10	I DEAD LOAD + I WIND LOAD X+ + I WIND LOAD Z+ UPWARD
Combination	11	I DEAD LOAD + I WIND LOAD X+ + I WIND LOAD Z- DOWNWARD
Combination	12	I DEAD LOAD + I WIND LOAD X- + I WIND LOAD Z+ UPWARD
Combination	13	I DEAD LOAD + I WIND LOAD X- + I WIND LOAD Z- DOWNWARD

Approved :-

Dr. ISHAQ MOBIN  
Professor  
Dept. of Civil Engineering  
Feroz Engineering & Technology  
Jalma Millia Islamic  
New Delhi-110025

### **3. STAAD MODEL:**



第10章