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*Preliminary Report on the Recent Event of Ground Subsidence at Joshimath,
District Chamoli, Uttarakhand*



By

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Abstract

Northmarch area experienced a sudden spurt in ground subsidence activity since 2nd January 2011 with severe damage in a large number of buildings that generated cracks, and failure of foundations. This ground situation compelled the local administration to temporarily shift many residents to safer places, along with engaging all scientific organizations and knowledge segments to immediately study and assess the situation.

Accordingly, services of the Geological Survey of India (GSI) were asked for by the Uttarakhand Government. Just the situation was further assisted by the Hon'ble Cabinet Secretary in the Government of India, Member Secretary, NDMK, and the Union Home Secretary. All organizations put forward their historical experience to understand the gravity of the problems and to search for the preliminary reasons for the such spurt in subsidence activity.

GSI was entrusted also to assess the hazard scenario along with an immediate task of assessing the feasibility of five identified rehabilitations also identified by the State Government. GD submitted its preliminary note of the feasibility of five rehabilitations also to the Uttarakhand Government on 18.01.2011, and also submitted its interim note to NDMK and CMU on the current demand status.

*This current preliminary report is the outcome of GSI's ongoing field studies in and around Northmarch area since 7th of January 2011 in search of finding the following facts after including ground evidence of distress, cracks, and also after evaluating all possible alternatives of causes and its impacts, including evaluating the current status of hazard in Northmarch area. The observations and inferences are made *a priori* facts around the following.*

- *To identify *a priori* facts the scientific reasons for such a spurt in subsidence activity since 2nd January 2011*
- *To review the geology and geomorphology of the area, and also to relate the current events with past evidence of such incidences in the area*
- *To potentially relate the ground cracks and subsidence with the proximal causal and triggering factors, and also understand its spatial and temporal character.*
- *To understand the extent of the damage if any in the Dwarka Heritage, and the areas in and around Northmarch area along with assessing its future vulnerability and risk*
- *To unravel the puzzle of strengthening the source of seepage water and also to understand the role of subsidence in assessing this slope stability problem.*
- *To examine the consistency of the Flood Risk Turner of the Dapruva-Flooded Flood project with the current extent of a hazard if any*
- *To suggest future studies and the way forward for better slope management and mitigation for Northmarch area*

After the above preliminary study, the following salient observations are made:

- **GSI identified and mapped 17 ground cracks** out of which 4 cracks are recent ones. (baked in the recent event of subsidence reported on 01.01.2023), the rest 13 cracks are older.
- The previous studies of GSI indicated that **owing to the presence of a thick, later unconsolidated debris mass on the Ashmath slope, the problems of experiencing creep, slumping, and subsidence in the Ashmath area are not the new ones, and they are being reported on many occasions in the past too during the last 4-5 decades**.
- The recent ground cracks mostly are aligned along a 10-15 m wide linear array trending NW-SE covering Jural Ghat, Manchar Bagh, Singapur, and Marwarli wards.
- The cracks are not concentrated in areas where heavy settlements are not present.
- Apart from the line sagging just near JP railway, no new sagging point having a record of muddy colored water was noticed in Ashmath. The particular line sagging point is located in Marwarli in the basement reaches of the linear array of the affected zone. The sagging discharge also reduced to a great extent from 800 LPM on 01.02.2023 to 17 LPM on 02.02.2023 suggesting the nature of any seepage water below or piping resulting in any further deterioration of slope stability status in the area.
- The subsurface studies by DMR and ITFC also confirmed the presence of **potential shallow subsurface water zone (12-15 m thickness) in Ashmath** the loose overburden material having direct connectivity to the new sagging point near JP railway in Marwarli, and this possibly leading to the current subsidence problems in Ashmath that are reported since 02.02.2023.
- Extensive run of piping action locally along a linear zone because of unconsolidated sagging zone within the Ashmath, coupled with an extensive mass loss of multi-layered soil structures on a narrow and potentially unconsolidated debris mass are the main reasons for lowering of shear strengths and increased shear stress of slope forming mass, which ultimately is the instrumental of the recent apart in subsidence and failure in some parts of Ashmath.
- From field, based on the spatial and temporal nature and pattern of ground cracks, it is highly unlikely that a large circular failure surface is in action in the area, however the same will be examined further during a detailed (1:5000) study to taken up further by DMR.
- **Because of the large intervening distance between the TEM tunnel and Ashmath Mass (more than 2 km), gross water-table in the levels of normal grade, and the nature of the sagging at Marwarli, the current activity of movement in TEM tunnel, much larger intervening gaps between EBM tunnel face and the ground of Ashmath Mass (more than 1 km), unattended water discharge amounts at the current TEM face, 10-5 m³ discharge observed on ground at JP railway sagging point on 02.02.2023 clearly substantiate that the recent sagging that started out near JP railway has no plausible connection with the NIT of Tagorewadi-Kalmugad Road Project of ITFC Ltd.**

1.0 INTRODUCTION

1.1 Background

The students and staff were alerted during reports of severe snake and facility damage due to activities in Lohardih, Chaibasa District, Chhattisgarh on 02.08.2022. Immediately on receiving a report from the Chhattisgarh State Disaster Management Authority (CSDMA), IIM, (present version of letter) Dr. Bhagendra Singh Naga, Director, Sri Satish Chandra Das, Sr. Geologist, Sri Taha Shrivastava, Geologist and Sri Shashank Kumar, Asst. Geologist (led by Mr. M.K. Kanishk, Director for carrying out fieldwork and collection of data on 03.08.2022). In addition to assessing the human resources in Lohardih, the CSDM team also carried out a preliminary assessment of five snakebite cases identified by the State Government on request. Subsequently, Dr. Harish Khatwaga, Director, IIM, Jaipur, was informed that the incident is a part of the effort from IIM by DR, IIT, IISc on 07.08.2022 onwards. Later, as advised by IIT, IISc, Dr. Prakash Jais, Dr. Bhagendra Singh Naga, Unit, Chhattisgarh was also notified at site to assess the situation on 14.08.2022. Because of the priority of rehabilitation of the affected people in Lohardih, as requested, IIM submitted its preliminary report on five pregnant snakebite cases to IIM Jaipur on 14.08.2022 simultaneously, continuing the field study in Lohardih to understand the current human situation and its possible reasons, and improve the site.

1.2 Study Area

Lohardih town in Chaibasa district, Chhattisgarh is a popular hill station and is a gateway to several Himalayan mountains having exceptional natural and phylogenetic values like Baidyath, Dhams and Bandhulal hills. The place also has strategic importance due to its proximity to the international border with China.

1.3 Location and Accessibility

Lohardih town is located on the left bank of **Ajandah river at Longitude E 83° 12' 39" 56'**; Latitude N 80° 21' 49" 52" N and falls in part of the survey of India Toposheet 52N/80. It is bounded by the Ajandah River in the north and west and by the Bhadgaon River in the east (Fig. 1.1) (Fig. 1.1).



Fig.1.2 Flood map of Jashnabadi town

1.4. Trail of rescue event

An incidence of severe ground subsidence took place at Jashnabadi town located in Chharoti district, Unnao district. The ground subsidence resulted in the development of several ground fissures and cracks in civil structures, including residential buildings, roads, bridges, etc. As per the reports in the print and electronic media and based on the account of the residents and the local administration, the present incident of accidental ground subsidence occurred in the even hours of 02:00:00, following a sudden gush of water from the first floor (Lat. N: 24.24.00N, Long. E: 79.19.42E, Elev: 140.7 m) of the cement slope in the campus of the colony of M/s Jashnabadi Associates Limited (JAL), located at Mayawati (which marks the lowest portion of Jashnabadi neighbourhood along the Ghaghara River). Subsequently, JAL authorities channelized the overflowing water into a drain for releasing the same at the road level. As per the data from the local administration, the total discharge of the subsided water was 600 litres per minute (LPM). Several households around Mayawati and in the upper stretches of Mayawati old Bazar Talua reported cracks in their buildings during the same time.

1.5 Previous Studies by GSI in Jaldhath Area

GSI has carried out geological and geomorphological studies in the Jaldhath area in the past on many occasions, and also carried out the spot-by-spot requests received from the State Government, local administration, and Military/paramilitary authorities on various issues related to slope stability problems. Hence, it is known (1999) established a thrust around that places a long history about "loss cover of Jaldhath trees (i.e. the 'Main Central Thrust') which separates most of the Central Orogenic to the north with the Garowal Group of rocks to the south. Latest later (1999), geologically mapped the area around Jaldhath in the Malakonda valley, and was the first to classify various lithological units separately. He marked the thrust in Malakonda valley at the same place as that of Davis and Gansser. Agarwal, N.C. and Mahapatra, S.K. (1974-75) carried out geological and geomorphological studies of the Jaldhath-Jaldhath area and stated that *Jaldhath is situated on a slope of semi-consolidated to semi-consolidated granite with large gneiss boulders*, which are seen in different parts of the area. According to them, probably, a large volume of rocks and loose material slides over the foliation planes of schists and gneisses due to instability of slope caused mainly by heavy cutting of Malakonda and Malakonda (Kumar and Agarwal (1983-84) and read the general geomorphological map of the area about the prevalent stability problems observed in the area. They have suggested possible measures to reduce the areas showing unstable conditions and suitable structural measures were suggested to control them. Prasad, et al (1987-88) carried out a geophysical survey at different locations of the Jaldhath-Jaldhath area to assess the stability problems. The gravity and magnetic volume surveys conducted by Gaurish Kumar, G.M.P. (1988, and A. J. (1991) have indicated that most of the area is covered by a matrix of loose formations like sand and clay with boulders. *According to them, the thickness of the loose formation in the Gaurish Kumar area is estimated to vary from about 40 m to over more than 100 m from the surface.* Prasad et al (2001-02) carried out landslide hazard analysis in the Jaldhath area and categorized the area into high, moderate, low, and very low hazard zones.

In addition to the above, GSI had also carried out conventional slope engineering geological investigations of the Tapovan Vishnupad Hydropower Project (1980s) for six field seasons from 1st 2001 to 2008 to 45, 2011 to 2014 and had submitted its reports to NTPC viz. Ghoshal B.C. and Dasgupta B. P. (2007 to 2008, Ghoshal B.C., Dasgupta B. P. and Dasgupta A. P. (2008 to 2009), Dasgupta A. P. and Dasgupta B. P. (2009 to 2010) and Dasgupta A. P. et al (2010 to 2011) etc.

2012), Hospital A. F. Hinganga Harish and Hospital B. K. (2012 – 2013), and Hospital A. F. Hinganga Harish (2013 – 2014).

In addition, Sh. V.K. Kanthia, Director, H.A. Ishikumbhari was a nominated member from GSE, Dehradun for the “Team of Experts” constituted by the Secretary, UIDRRA, on 26th July 2022 to study the reported subsidence in the Indrawati area. On the basis of ground subsidence in and around Indrawati area, Sh. Kanthia visited the area along with other team members between 16 and 19 August 2022 and submitted his report.

The previous studies, quite indicated that the study area is and around Indrawati Town has been experiencing recurrent problems of slope stability in the form of subsidence, sliding, slump, failure, landslides, etc. resulting in damage to several buildings for a very long (for the last 4-5 decades or so). In the year 1976, the States Commission (1976) also submitted a detailed report outlining some of the causes of the instability problems observed in the Indrawati area and also suggested relevant remedial measures.

1.4. Scope of present study:

Immediately after the recent incidence of severe ground subsidence reported on 02.08.2022, the Uttarakhand State Disaster Management Authority (USDMA) requested GSI for carrying out a geotechnical assessment of the site, about 500 m from the temporary rehabilitation site lower on (16.00.26196N, 102° (2022), dated 07th January 2023 and 1725.75196N, 100° (2022) dated 10th January 2023. Subsequently, the National Disaster Management Authority (NDMA) advised all central agencies working at Indrawati to study, prepare, and submit a preliminary report on the current hazard scenario of Indrawati within a month, followed by the submission of a detailed study report subsequently. In this line with this advice, a request for carrying out large-scale geological mapping (preferably at 1:20,000 scale) of the Indrawati town was also received by GSI from the Additional Chief Executive Officer, USDMA, Dept. of Uttarakhand via letter no. 1701/NDMA-1194 (2023) dated 16th January 2023.

While working at the site as directed by NDMA, the GSI team has already submitted a preliminary note titled “Preliminary Note on the Appraisal of the Hazard Scenario at Indrawati, District Chamoli, Uttarakhand” on 16th Jan 2023 to USDMA-NDMA. In addition, the GSI team completed its preliminary assessment of the site (subsidence site) and has already submitted a

now titled "Note on the Preliminary Geological Assessment of the Proposed Tarpuray Mainline Line around Jodhpur Area, District Chhattri, Uttaranchal" on 18.01.2021 (Annex 2).

To understand prima-facie the scientific reasons for the recent spurt in ground subsidence activity in Jodhpur this was reported on 02nd January 2021, the GSI team carried out a general geological assessment of the subsidence, damage to the buildings, roads, and other infrastructures in and around Jodhpur area between 19th and 19th January 2021. A detailed summary of recent/supposed ground cracks has been prepared. The seepage from the recently-appearing seepage point was monitored and the daily update regarding the discharge of water from this point was gathered from the local administrative authorities. The GSI team also collected various (open) drawings, design drawings, the geological section along HRT, progress of excavation at different HRT faces, and data on seepage at different faces of HRT (CRM) cracks from the authorities of NIPCC Ltd. Jodhpur's. Based on these records, issues about the occurrence of HRT cre. is thus inferred (damage due to excavation of HRT, if any, or seepage of water from the present face of HRT with TMS, and it's possibility of seepage from the HRT (HRT) face to the HRT (HRT) face have also been preliminarily examined and an engineering geological appraisal has been prepared and included in this report as a separate chapter.

The preliminary (open) issues already submitted to GSI by other relevant agencies like MoEF and UYWB have also been duly consulted for getting the information on subsurface geological and hydrogeological conditions, and forming a logical and coherent scientific viewpoint based on their inputs re-visit GSI's ground observations.

2.0. REGIONAL GEOLOGY, GEOMORPHOLOGY AND SEISMOTECTONIC SET UP

The study area is located within the frontal thrust exposure of the active Himalayan fold thrust belt (FTB). This part of the Himalayan FTB is characterised by several linear seismic zones fragmenting into numerous narrow slices of varying dimensions and it is bounded by the Main Boundary Thrust to the south and Main Central Thrust Zone (MCTZ) to the north. The broad regional geological map of the area is depicted in Table 1. The area comprises rocks of the Central Ophiolite Group which are thrust over the Garkwa Group of rocks separated by the Chaurali and the Piplitoli formations, along a southward dipping major tectonic discontinuity known as Main Central Thrust (MCT) (Fig. 2.1).



Fig. 2.1: Regional geological map of the area showing MCT

2.3. Hiding Formation: This formation is represented by quartz mica (yellow) siliceous conglomerate, quartzite, and minor mudstone and basic rock. The Hiding Formation comprises the Chandi formation of the Gachal Group of rocks near Hiding village along the northward dipping M.T. Intense cooling associated with hiding has been reported by earlier workers along the river.

2.4. Gachal Group: The Gachal Group of rocks is represented by Pyralite and Chandi formations. Pyralite formation comprises quartzite, calc-silicate, limestone/dolomite, minor conglomerate, talc, etc. and the latter comprises Quartzite, siliceous schist, basic rocks, limestone/dolomite, and minor magmatic. These low-grade metamorphites are exposed in a series of antiformal synclinal structures around Pyralite.

2.5. Geomorphology:

The Akashnadi River, flowing on one of 1:1.11 sq. km. is the main river in the area, which originates from the Sanparikh-Bhagabhai-Chandi group of gneisses. The major tributaries of Akashnadi are (Bhariganga, Pindiganga, Nandolam, Mandhara, and Pradon). The area is covered by a closely spaced network of channels, tributaries, and streams fed by spring water. The drainage of the area is mainly controlled by biological structure and tectonic factors. Higher-order streams have both tectonic and biological control while the lower-order streams have developed on the local tectonic upliftments etc. The Akashnadi river (Akashnadi flows increases in the structural state in a deeply incised channel with regularly terraced patches of (barren) (granitic and sandy) deposits along its path.

Most of the geomorphic features are due to polycyclic, endogenic, and exogenic processes of varying intensities. The valleys are deep, steep, and narrow and the channel gradient is higher. The channel material mostly comprises pebbles, cobbles, and boulders in a sandy matrix moving over a rocky floor. The alluvium consists of boulders, cobbles, and pebbles covered with sandy clay, and thick sand horizons are present in the river banks. The left slope on either side of the river from high rugged smooth surface due to erosion and this is both in and above. The geomorphic cycle is in the youthful stage and hence the denudational processes are operating at a faster rate with the highest energy. The present landscape represents the complete process of denudation under the influence of thermal conditions, later modified by recent modifications. The glacial deposits and glacial facies deposits occur near Vijaynagar and Indirapuram.

1.5. Slope Classification:

Altitude is classified as a slope of semi-mountainous to mountainous areas with large grassed bushes, which are observed in different parts of the area. **The elevation of a climatic zone varies from 1312 m to 1329 m with a maximum altitude near Ash and a minimum at the Akhmatu River bank.** Maximum area is 40% falls within the slope class of 20°-35° whereas a substantial part i.e., 19% of the slopes are steeper than 40°. From Akhmatu River to Ash there are multiple slope breaks characterized by steep slopes close to the edge of Akhmatu River for a length of about 30 km is there which the slope is comparatively gentle with 24° - 35° gradient in the lower middle part i.e., on the slopes below hanging mouth. Mountain high and Singhar area (Fig. 1.2, Photo 1.1). The area around Manchar high and Upper Ghara is nearly flat whereas the slopes become steeper towards Ward Gura and further up towards Ash. The dominant slope aspect is towards the North (Fig. 1.10) i.e., towards the Akhmatu River.

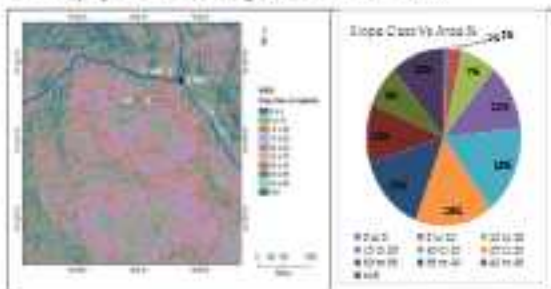


Figure 1.2 Slope map and distribution of slope angle classes

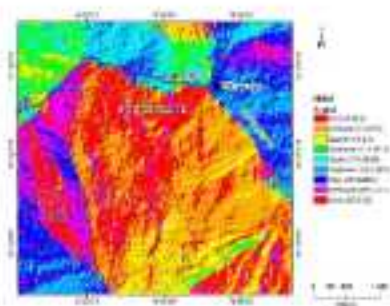


Figure 2.1 Map of the topographic map of Indus valley



Photo 2.1 View of Hume Fort of Indus valley (a Mountain Top)

2.3. Drainage Characteristics

On the eastern part of Jaldimach area, two main perennial streams are present which are S.T. river and Charan Ndi. Flowing northwesterly into the Haldipaga River, where another perennial stream Jagdhat joins in the western side of Jaldimach flows towards the NNW direction and joins Haldipaga River. There are several springs within Jaldimach area (Fig. 2.4).



Fig.2.4. Drainage map showing the location of springs in Jaldimach area.

Two streams flowing towards the north and draining into the Haldipaga River are observed at both the eastern and western sides of the Narayn Temple. The topographic expression of these two streams is very clear even in the Google Earth's image (Fig. 2.5). Slope direction is towards the northwest in the eastern part, the north direction in the central part, and NNW in the western part of Jaldimach area.



Figure 2.3 Google Earth image of Ayathnathi area

Below Jeeknathi, the Hilsaht Ganga joins Ayathnathi at Yathnagrap and upstream of this confluence, the Hilsahtnathi flows through a steep gorge (Fig. 2.4) of about 500 m in depth. Valley walls are mostly low covered, however, at places, the inclination of the yellow “U” form of the valley can be easily observed, as near Yathnagrap at an altitude of about 200 m. The river runs across mainly quartzite, schists, and gneisses (Figures 4) and 5) of Sengupta (1974, 77).

2.8 Seismotectonics:

Ayathnathi basin is situated on the hanging wall side of the Main Central Thrust (MCT). The epicenters of the main and aftershocks of the seismic events of the Garo-Khadi Earthquake of 20 October 1911 and the Chomol Earthquake of 25th March 1997 show clustering close to MCT (Kawada et al. 1997; Wada et al. 2003). The Ayathnathi area falls within seismic Zone V of Indian Seismic map of India (GSI 2002) which indicate high seismic sensitivity with earthquake events $> 1 M$.

2.9 Local geology around Jeeknathi area:

The study area is occupied by rocks of the Hilsaht and Main formations of the Central Gneissic Group, Main formation (Proterozoic age) covers a major part of the area and

embedded in the ground. Shallow portions of this micaceous sand, silt, and clayey material having low cohesion. A few scattered outcrops are exposed below the Marwar area on the left bank of the Mahanadi River and between of Purni village. Erosion cut of water in the form of natural gorges has been noticed at several locations along the slope which indicates that the overburden material is composed with shallow soil surface water often, aspect is being constantly dealt with in chapter 4 later).

The highly consolidated and homogeneous mass of yellowish and reddish debris consists of large to very large-sized rock boulders of granitic gneiss and schistose gneisses embedded in a matrix of sand, silt and clay size particles having very low cohesion. Signatures of delving of slopes and caving of ground have been noticed in the overburden covered slopes in and around Indrakant, which is indicative of the fact that this area has been experiencing incidences of ground subsidence in the past as also affirmed by the previous studies conducted by IRI on several occasions in the past, viz. Agarwal, N.C. and Bhaskarpal (pg. 5-8) (1974-75), Kumar and Srivastava (1985-86) and Prasad, et al (1987-88).

3.3 NATURAL SPRINGS AND DEEPADE CHARACTERISTICS OF DEWLY APPEARED SOURCE

The entire landscape area from Station A to station B is covered with naturally occurring springs. These springs are used as a source of potable water by the locals. The locations of the naturally-occurring springs are given in Fig. 2.4 and their details are given in table 3.1. *The distribution of naturally occurring springs indicates that lower unconsolidated strata were occupying the slope, over which Andamati Township is situated, is already saturated with subsurface water. Furthermore, the occurrence of spring at different levels indicates the location where the natural topography has intersected the free flow of subsurface water (Fig. 2.4, Table 3.1, Photo 3.1 to Photo 3.4). This also indicates the presence of perched shallow water bodies within the overburden material. The water coming out from the natural springs is however clear and is being utilized by the residents for potable use.*

Table 3.1 Location of Natural Springs in Andamati

#1	Name	Lat (N)	Longitude (E)	Elevation (m)	Nature of Water	Remarks
1	Natural Spring	90.1597	79.1540	1191	Clear	Intercepting Natural water table
2	Natural Spring	90.1579	79.1540	1080	Clear	Intercepting Natural water table
3	Natural Spring	10.0231	79.1546	1186	Clear	Intercepting Natural water table
4	Natural Spring	90.1596	79.1532	1275	Clear	Intercepting Natural water table
5	Natural Spring	90.1542	79.156	1013	Clear	Intercepting Natural water table
6	Natural Spring	90.1611	79.1616	1048	Clear	Intercepting Natural water table

3	Natural Spring	98.118	98.000	100%	Clear	Intercepting Natural water table
4	Natural Spring	98.1110	98.2740	100%	Clear	Intercepting Natural water table
4	Natural Spring	98.1750	98.1740	100%	Clear	Intercepting Natural water table
10	Natural Spring	98.1470	98.1540	100%	Clear	Intercepting Natural water table
11	Natural Spring	98.1610	98.1550	100%	Clear	Intercepting Natural water table
12	Natural Spring	98.1480	98.1540	100%	Clear	Intercepting Natural water table
13	SP-2 Spring outcrop	98.1580	98.1550	100%	Minutely	Not a regional flowage on EL. 98.1625

The field inspection notes by C&I in Maricao, Sanjahan, Maricao high, Maricao middle, Maricao, Sanjahan, and Arik were indicated that at several locations, the water from the natural springs is seeping into the ground (Photo 1.4) and this seeping the overburden were over which the different retention of that result occurred.



Photo 1.1 Natural Spring (Sanjahan high)



Photo 1.2 Natural Spring (Sanjahan)



Photo 1.1 Natural spring at Haranoph Masole



Photo 1.4 Natural Spring above Masole gate (water seeping into the ground)



Photo 1.3 New Spring point at P. Celina Masole



Photo 1.6 Destroyed Spring outlet at the gate of St. Tikhon's Masole

The information gathered from the field site suggests that the construction of the well in the area has not only put that area not connected to the sewerage system at the sewerage treatment plant of Jermuk. Therefore, the domestic sewerage water of Jermuk is also contributing substantially to the recharging of contaminated groundwater zone, adding water to the

underground water reserves.

3.2. Characteristics of New Seepage point at JP Colony Marwar

The water coming out from the newly appeared seepage point near JP colony in Marwar is alkaline and muddy and called effluent (Fig. 3.2 and Photo 3.1), suggesting existence of severe piping of fine material with such seepage. After the tracing of water from this point, the authorities of JP disconnected this water into a hand channel through which it is being released at the road level (Photo 3.3). It is interesting to note that apart from this, there have been no new seepage points observed/ reported anywhere else in the Indira Park application area. The discharge of water from the seepage point at JP colony (Lat. N 30.564896, Long. E 75.553494, GDA 1987) is estimated from 500 LPM (2017-2021) to about 17 LPM per 02.10.2021 as per the daily update of Jawahar Narmada management Bulletin issued by District District Management Authority, Chaudhri (<https://www.dma.gov.in/jawahar-narmada>). It is worth mentioning here that in the previous survey as the stated presence of continuous, sustained by GDA in 2019/20, as 10.10.2021 it was pointed out that this new seepage point at JP colony, Marwar, may be from a perched water body located at the discharge area within the overburden. The subsequent gradual decrease in the discharge of water from the newly appeared seepage point at JP colony Marwar has further corroborated the above inference by DM.

3.3. Geological Assessment

Field traverses and ground geological observations by GSI also revealed that the recently developed ground cracks in the Indira Park (those reported on 02.01.2021 and later) are aligned along a linear away along NNEW – direction, which may also indicate a trend showing weak zone of ground connection too. Later, the subsurface probing (by 30000 and 60000) magnetically established the presence of such shallow zones of sub-surface saturation (perched water bodies) within the track area as below and highlighted within the map. These observations will however be studied and validated during detailed geological mapping by GSI subsequently.

3.4. Use of studies by Central Ground Water Board (CGWB)

The Central Ground water board (CGWB) has carried out subsurface geophysical explorations through Electrical Resistivity (ER) in different parts of Indira Park covering Park Canal, 22 / 12

Mamari High, Singhar and Mamari areas. They have also studied the discharges and yield from the three hand pumps located in a linear array between Mamari, Singhar and Nand Chaur. The recent studies of the yield from the installed three hand pumps by Central Ground Water Board (CGWB), in and around Nand Chaur area, have concluded that "Conservation of all these in the same zone may indicate that area associated with the ground water may prove to be an subsidence". The findings of CGWB to put their reports also confirms that "The depth of water level in three hand pumps ranges between 12 to 45 m. Daily depth of water level has been monitored and it is observed that water level is declining in Singhar area, with average rate 25-30 cm per day, while in other hand pumps, the water level is almost constant. The decline in Singhar hand pump indicates that water bearing strata is tilted in the land subsidence. Land subsidence may be extended in and around Singhar Hand pump" (Source: CGWB report submitted to CWS).

3.3. Conclusion on hydrological point that its surface observation:

- The existence of natural springs at several locations is a clear evidence that the slope forming heterogeneous strata material is saturated with water at different levels. In addition, the saturation level of the soil mass, by other means such as surface phenomenon or otherwise, results in the building up of structural pore pressure in the soil mass back of the slope face. This, in turn, gives rise to an adverse condition for the stability of the slope, as the water saturation increases the shear strength of the slope-forming material.
- The outcomes of the study by CGWB have brought us a view of saturation in a linear pattern between Mamari, Singhar and Nand Chaur and corroborates the linear trend (NW-SE) of all the structures joined out by GDS from ground observations, and the same sense of saturation with groundwater is found continuous with the linear arrangement of development of recent ground cracks. The above findings led by CGB and CGWB, thus corroborate that the newly-appeared seepage from a IP colony which had no water discharge of about 60 LPM, or of 32.31 GPD, is prima-facie connected to a perched water body that is located within the block in the immediate vicinity. The piping of this material by the newly developed seepage has been continuous in its seeping when ground cracks subsidence is sensed 'beneath' creek structures that are aligned along a linear array meeting NW-SE, affecting parts of Mamari, Singhar, Nand Chaur, etc.

- The currently affected area is primarily under habitation and the recently developed ground cracks are also developing in its vicinity only. The adjacent areas where there is either scarcity of no habitation have so far not been affected by the recent (03-04-2023) event of ground deformation.
- **Continuation of subsided buildings were even higher than ever the type of loose, uncompacted, and differentially compacted subsoil, material having lower shear strength which retarded the driving force on the slope that triggering about the threat of sliding and hence leads to the ground distress and failures.**
- It is also worth mentioning that there is no reporting of new ground cracks in the backwash area for the last 11 days (14.01.2023 to 31.01.2023), which also confirms the view reported by CGO (then appointed as MDA, on 06.11.2023) wherein it was inferred that decrease in the discharge from the newly appointed seepage point will aid in reversing the position of ground subsidence. As reported by the local administration, the discharge of 03.02.2023 from the HP Colony seepage point is 17 LPM only, and the water level out is down, which indicates that piping has already reached its waning stage, which will a positive sign for an increase in down strength and stability of the slope facing cracks present there.

4

4.1 GROUND DEFORMATIONS AND DRESS FEATURES

The recent event of the emergence of a new sewage pipe at BP colony at Marwar on 02-04-2023 has gathered more of the attention of the community and it has been widely covered in the media and an inspection has been carried out at the ground distress in Indira Park area (see photograph after 02.04.2023). It is up to mention here that the ground subsidence phenomenon of different degree in the Indira Park area had been a chronic phenomenon for more than the last half decade and the same is already understood and mentioned by the earlier reports of IAS viz. S. J. Patel, N. K. and Shrivastava, S. K. (1974-75), Kumar and Srivastava (1985-86), Prasad, et al (1997-98) and also in the report of M.C. Shukla (committee of 1974).

There were reports of ground cracks in some parts of Indira Park in the year 2012 and along the perimeter of it, the Uttarakhand Administration constituted a team of experts drawn from various organizations including IIS in August 2012 and the team submitted a report to the Government of Uttarakhand on the preliminary findings. The Uttarakhand State Disaster Management Authority (USDMA) convened a meeting subsequently along with a team of experts and various other departments to find practical solutions to the existing problem, and in that meeting IIS highlighted the need for documentation of the existing situation in the entire area to assess the height of the risk constructions and also to monitor the ground movement to know some potentially vulnerable to ground deformations. Subsequently with the sudden emergence of a new water source, on 02-04-2023, at the top level of the BP colony at Indira Park, Marwar, Indira Park, the USDMA, on 02.04.2023, notified the team of experts constituted earlier (in August 2012) to assess the ground situation. The team visited the Indira Park area on 05-04-2023 and submitted the joint report with initial findings to the Government of Uttarakhand on 06-04-2023.

4.3 Site Observations

The AIC team working on the ground continued to collect data on the ground cracks and took measurements (average width of the crack mouth of each crack (Photo 4.1)) to understand the stress under the maximum impact. Based on the field observations and the characteristic features of different ground cracks, USA categorized the ground cracks into two categories, i.e. i) the recent or new cracks those which are directly related to the event of 02-01-2021 (Photo 4.2, 4.3, and 4.4) and ii) Old ones that predate the event of 02-01-2021 (Photo 4.5, 4.6 and 4.7). Out of the total recorded 88 ground cracks (88/21/02/21), 79 cracks are found as recent ones related to the event of 02-01-2021, while the rest 42 are the older ones. The spatial distribution of the ground cracks is depicted in Fig.4.4 and the parameters detail of all the ground cracks is shown in Table 4.1 below:



Photo-4.1 – Important Locations of Antsiraha (RI) – River Caen; (CM) – Ajyze Water; (NH) – Nanahy Island; (CM) – Camille Mader; (NH) – Maurice Rigo; (SC) – Saint Louis; (SI) – Saint Elie; (MW) – Maroni; (PW) – Palace



Fig. 1.1 Distribution of Ground Chicks (M and N) in various states

From Table 4.1, it is evident that out of the 41 ground chicks, 27 numbers of chicks are available and the number of chicks are more and out of 44 numbers of forest ground chicks, 12 chicks are available in forest whereas, 27 numbers of chicks are forest chicks (Fig. Chart 4.2A to 4.2C).

Table 4.1 Parameters Details of Ground Chicks in Different part of India as in

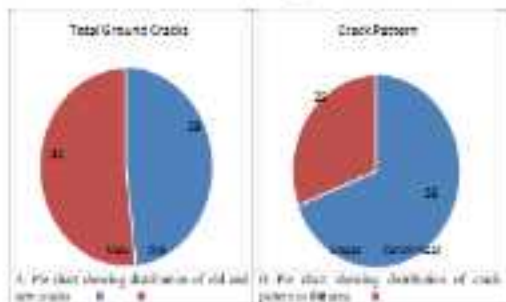
Sl.	Latitude	Longitude	Old/ New	Observation	Type
1	81.3771	71.355528	New	200	Cardinal
2	81.3770	71.355444	New	200	Cardinal
3	81.3773	71.355522	New	200	Cardinal
4	81.3774	71.354444	New	200	Cardinal
5	81.3777	71.355522	New	200	Cardinal
6	81.3776	71.355444	New	200	Cardinal
7	81.3778	71.355522	New	200	Cardinal
8	81.3779	71.354444	New	200	Cardinal

9	SL37181	FL3764880	New	200	Carbide
10	SL37181	FL3764881	New	200	Carbide
11	SL37181	FL3764882	New	200	Carbide
12	SL37181	FL3764883	New	200	Carbide
13	SL37181	FL3764884	New	40	Synthetic
14	SL37181	FL3764885	New	140	Synthetic
15	SL37181	FL3764886	New	140	Synthetic
16	SL37181	FL3764887	New	140	Synthetic
17	SL37181	FL3764888	New	140	Synthetic
18	SL37181	FL3764889	New	140	Synthetic
19	SL37181	FL3764890	New	140	Synthetic
20	SL37181	FL3764891	New	140	Synthetic
21	SL37181	FL3764892	New	140	Synthetic
22	SL37181	FL3764893	New	140	Synthetic
23	SL37181	FL3764894	New	140	Synthetic
24	SL37181	FL3764895	New	40	Synthetic
25	SL37181	FL3764896	New	40	Synthetic
26	SL37181	FL3764897	New	40	Synthetic
27	SL37181	FL3764898	New	40	Synthetic
28	SL37181	FL3764899	New	40	Synthetic
29	SL37181	FL3764900	New	40	Synthetic
30	SL37181	FL3764901	New	40	Synthetic
31	SL37181	FL3764902	New	40	Synthetic
32	SL37181	FL3764903	New	40	Synthetic
33	SL37181	FL3764904	New	40	Synthetic
34	SL37181	FL3764905	New	40	Synthetic
35	SL37181	FL3764906	New	40	Synthetic
36	SL37181	FL3764907	New	40	Synthetic
37	SL37181	FL3764908	New	40	Synthetic
38	SL37181	FL3764909	New	40	Synthetic
39	SL37181	FL3764910	New	40	Synthetic
40	SL37181	FL3764911	New	40	Synthetic
41	SL37181	FL3764912	New	40	Synthetic
42	SL37181	FL3764913	New	40	Synthetic
43	SL37181	FL3764914	New	40	Synthetic
44	SL37181	FL3764915	New	40	Synthetic
45	SL37181	FL3764916	New	40	Synthetic
46	SL37181	FL3764917	New	40	Synthetic
47	SL37181	FL3764918	New	40	Synthetic
48	SL37181	FL3764919	New	40	Synthetic
49	SL37181	FL3764920	New	40	Synthetic
50	SL37181	FL3764921	New	40	Synthetic
51	SL37181	FL3764922	New	40	Synthetic
52	SL37181	FL3764923	New	40	Synthetic
53	SL37181	FL3764924	New	40	Synthetic
54	SL37181	FL3764925	New	40	Synthetic
55	SL37181	FL3764926	New	40	Synthetic
56	SL37181	FL3764927	New	40	Synthetic
57	SL37181	FL3764928	New	40	Synthetic
58	SL37181	FL3764929	New	40	Synthetic
59	SL37181	FL3764930	New	40	Synthetic
60	SL37181	FL3764931	New	40	Synthetic
61	SL37181	FL3764932	New	40	Synthetic
62	SL37181	FL3764933	New	40	Synthetic
63	SL37181	FL3764934	New	40	Synthetic
64	SL37181	FL3764935	New	40	Synthetic
65	SL37181	FL3764936	New	40	Synthetic
66	SL37181	FL3764937	New	40	Synthetic
67	SL37181	FL3764938	New	40	Synthetic
68	SL37181	FL3764939	New	40	Synthetic
69	SL37181	FL3764940	New	40	Synthetic
70	SL37181	FL3764941	New	40	Synthetic
71	SL37181	FL3764942	New	40	Synthetic
72	SL37181	FL3764943	New	40	Synthetic
73	SL37181	FL3764944	New	40	Synthetic
74	SL37181	FL3764945	New	40	Synthetic
75	SL37181	FL3764946	New	40	Synthetic
76	SL37181	FL3764947	New	40	Synthetic
77	SL37181	FL3764948	New	40	Synthetic
78	SL37181	FL3764949	New	40	Synthetic
79	SL37181	FL3764950	New	40	Synthetic
80	SL37181	FL3764951	New	40	Synthetic
81	SL37181	FL3764952	New	40	Synthetic
82	SL37181	FL3764953	New	40	Synthetic
83	SL37181	FL3764954	New	40	Synthetic
84	SL37181	FL3764955	New	40	Synthetic
85	SL37181	FL3764956	New	40	Synthetic
86	SL37181	FL3764957	New	40	Synthetic
87	SL37181	FL3764958	New	40	Synthetic
88	SL37181	FL3764959	New	40	Synthetic
89	SL37181	FL3764960	New	40	Synthetic
90	SL37181	FL3764961	New	40	Synthetic
91	SL37181	FL3764962	New	40	Synthetic
92	SL37181	FL3764963	New	40	Synthetic
93	SL37181	FL3764964	New	40	Synthetic
94	SL37181	FL3764965	New	40	Synthetic
95	SL37181	FL3764966	New	40	Synthetic
96	SL37181	FL3764967	New	40	Synthetic
97	SL37181	FL3764968	New	40	Synthetic
98	SL37181	FL3764969	New	40	Synthetic
99	SL37181	FL3764970	New	40	Synthetic
100	SL37181	FL3764971	New	40	Synthetic

40	SL3342	SL738627	184	201	Cardinals
41	SL3376	SL732944	184	201	Cardinals
42	SL3362	SL732222	184	200	Cardinals
43	SL3407	SL738022	184	200	Cardinals
44	SL3330	SL737947	184	201	Cardinals
45	SL3336	SL737361	184	201	Cardinals
46	SL3367	SL732360	184	201	Cardinals
47	SL3475	SL738976	184	200	Cardinals
48	SL3354	SL735029	184	200	Cardinals
49	SL3609	SL7609	184	201	Cardinals
50	SL3636	SL767644	184	200	Cardinals
51	SL3637	SL766467	184	200	Cardinals
52	SL3616	SL765022	184	200	Cardinals
53	SL3472	SL765026	184	140	Snaps
54	SL3426	SL767644	184	140	Snaps
55	SL3407	SL767644	184	156	Snaps
56	SL3334	SL7666197	184	40	Snaps
57	SL3281	SL733000	184	140	Snaps
58	SL3472	SL736222	184	11	Snaps
59	SL346	SL764022	184	40	Snaps
60	SL3462	SL764276	184	40	Snaps
61	SL3476	SL748899	184	20	Snaps
62	SL3476	SL760776	184	147	Snaps
63	SL3367	SL73802	184	64	Snaps
64	SL3486	SL767276	184	20	Snaps
65	SL3462	SL764269	184	50	Snaps
66	SL3476	SL7676	184	50	Snaps
67	SL3330	SL7379467	184	156	Snaps
68	SL3407	SL748911	184	40	Snaps
69	SL3336	SL737361	184	60	Snaps
70	SL3332	SL738000	184	20	Snaps

11	0.1062	71.000000	0.04	20	Sample
12	0.1111	71.000000	0.04	11	Sample
13	0.1020	71.004544	0.04	10	Sample
14	0.1490	71.070000	0.04	40	Sample
15	0.1012	71.000000	0.04	40	Sample
16	0.1170	71.070000	0.04	40	Sample
17	0.1060	71.000000	0.04	11	Sample
18	0.1110	71.000000	0.04	140	Sample
19	0.1000	71.070000	0.04	50	Sample
20	0.1000	71.070000	0.04	40	Sample
21	0.1011	71.000000	0.04	11	Sample

Another interesting fact that emerges from Fig. 4.1 is that the majority of the recently appeared ground cracks from Suez Canal to the Suez area aligned along a linear array trending NWW - SSE (Fig. 4.1; Fig. Chart 4.20). The other set of ground cracks are aligned in E-W/NNE - ESE direction and are concentrated in Matruh (Fig. and Fig. Chart 4.21).



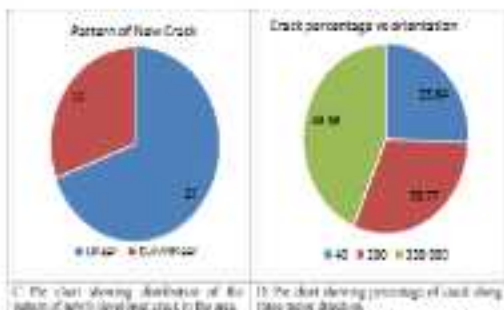


Figure 4.2 Pie Chart Distribution of Ground Crack in Aramco Area

FIELD PHOTOGRAPHS



Photo 4-1 **Vertical** Ground Crack at JP Colony (Marine)



Photo 4-2 **Horizontal** Ground Crack at JP Colony (Marine)



Photo 4-3 **Horizontal** Crack in the Open Ground of a house in Aramco area



Photo 4.2: Fld Crack on the floor of the house of Mr. Subari at Nandipati.



Photo 4.3: CMV Crack in the house near the Subari house at Nandipati.



Fig. 4.1: Fld Ground Crack in Nandipati.

In spite of the 39 rain events, 38 are in the Marathahalli area. 27 are in Bangalore and south are in the Varanasi area. The orientation of the cracks shows that there are transverse cracks parallel to ground contours. The majority (44%) of these recent ground cracks are aligned NNEW - SSW, and another 37% of these recent ground cracks are aligned WNW - ESE. Near Marathahalli to Bangalore and Mandarbhog area, whereas the remaining 27% of the recent ground cracks are aligned NW only and they are concentrated in Bangalore south only (Fig. 4.1). There is a total of 31 cracks recorded in the area during which 62 cracks are old cracks and 39 cracks are newly developed cracks. The study of the cracks indicates that the recently appeared ground cracks are mostly concentrated in 4 roads i.e. Marathahalli road, Bangalore road, Mandarbhog road and Nandipati road.

4.2. Geological Assessment

In attempt the team made to categorize the reported cracks to get a realistic appraisal of the damage or extent of ground distress. It has been found that many of the reported cracks are surface and do not have penetration into the ground hence they should not be considered a part of the ongoing distress following the incident of 15.06.2013. The two reported cracks as shown in Photo 4.2 and Photo 4.3 are only confined to the track trolley 1st pit which may be a result of improper laying of the aggregate beneath. There is no continuation of such cracks in the open

present at the margins which indicates that this is not related to the ground subsidence.



Photo 6.7 Crack in the thick top layer of Road near Yotat Givat



Photo 6.8 Crack in the thick top layer of Road in Sdeh area

It is pertinent to mention that the ground distress and damages to the structures are confined within a 10 – 60 m wide strip across the point from where the new coverage has appeared in the JV colony (Photo 5.7) and the buildings' structures, beyond this zone, even in JV colony have not been affected by any ground movements. From this point (Photo 5.7) the frequency of the recent ground cracks follows a trace along meeting NWN – SEF joining Sdeh area, Ma'abar High, and Sdeh area. The other distinct pattern of recent ground cracks, trending E – W, is parallel to the coastline and is confined to the inner edge of the flat terrain surface at the Ma'abar High area.

The density of recent ground cracks is more in the area which are already built and dotted with several buildings, some of which are multi-storied (Photo 6.9). This has been the case with the area like **Ma'abar High and Sdeh area, where the occurrence pattern of ground cracks have been recorded and damages to the structures has been reported.** The heavy load exerted by a dense construction of housing structures, over the heterogeneous colluvial/silt/clayey debris mass, which is saturated with shallow subsurface water, only accentuated the above issues on the slope thus accentuating the subsidence in these areas. Further strengthening support has been noticed at the Sdeh area where the houses are constructed over a raised platform made up of random rubble in dry masonry (Photo 4.10). The vertical and horizontal load, exerted by the multi-storied building above the pile bases (Photo 4.11), was possibly more than the load

bearing capacity of the raised platform over which the park house was constructed.



Photo 4.8 Densely packed construction with many buildings



Photo 4.9 Climb on the Paved Platform below Park House in Sogdiana area

4.3 Data of Seismic by NGR

As mentioned in the NGR report (as received from CSRI) “A preliminary DEMSAR analysis on the European Space Agency’s satellite Sentinel-3 SAR images show the LGO (line of right) ground subsidence of the order of ~ 10 cm between December 2022 and January 2023”. The plot of numerous vertical subsidence and -thrusts as per the DEMSAR analysis, is aligned NW – SE (NGRI report dated 21-11-2023) further, the plot of physically mapped fault displacements as in the Latest plot of GPR profiles of NGRI (NGRI report) shows that the ground cracks between Jinn (Jinn and Maohai) high areas are aligned roughly NW – SE (NGRI report dated 21-11-2023, as received from CSRI). The findings of DGRC’s drone-mounted video-aerial pointing also aptly corroborate the ground geological observations by GPR that majority of the recently appeared ground cracks are falling in a linear array aligned NW – SE direction.

4.4 Probable Causes of Recent Ground Subsidence

- As mentioned earlier, the entire slope between Hall and Alimarkhi River is and around Asmullah is occupied by a thick deposit of alluvium and old sand dunes (Photo 4.11) which is heterogeneous and is characterized by very long-crested rock boulders (A12) embedded in a fine matrix of sand/silt/clayey material.



Photo 4.11 View of slope occupied by Old Akadava natural



Photo 4.12 Front view of Earth Vender (Approaching for soil)



Photo 4.13 House rising over the old slide debris



Photo 4.14 Removal of tree roots resulting in the flow below the giant tree

- The presence of springs at several locations on the slope between Aki at the top and Akadava River at the bottom (Fig. 2.6), clearly indicates the presence of shallow subsurface water flow from soil perched water bodies within this large of old slide debris mass. In the prevailing ground conditions, the placement of ground subsidence and creep are common, and the same is usually properly monitored.
 - The removal of trees occurred from the mass eroding (by the shallow subsurface water flow) due to deepening the surface precipitation in which the great weathering is established.

results in the creation of hollows or void spaces beneath the joint holders (Photo 4.13). More and more material of finer material, next a long period because of piping, results in the vertical settlement of joint rock holders under the influence of gravity. The settlement of each joint holder causes a sagging on the surface, particularly when the settlement of holder is happening close to the ground profile. This settlement is reflected in the form of ground fissures or cracks, which in the initial stages may be very thin, but with the continued process, they may show progressive widening with time. In the Ardara area, it is common to notice the construction of houses over the slope having thick bedded lava's or rhyolite material (Photo 4.14). Any such piping action is described above as you find ground cracks that are severely tilted and are diverging with piping cracks at building walls and floors in such cases. This has been the phenomenon, commonly observed in the Ardara area, in the last 4 to 5 decades (Refer to earlier reports of GSI and the report of the Studies committees). However, such migration of ground fissures will be much lesser in the areas which are not heavily loaded with multi-storeyed heavy civil structures.

- i. The recent onset of accelerated widening reported in 2021/2022 observed in some parts of Ardara, is a result of a combination of two or three factors. The heterogeneous old slide debris is inherently poor in cohesive strength, the settlement of it by shallow subsurface water only acts in reducing the shear strength of this material further. In addition, the increasing vertical load imposed by a dense network of multi-storeyed buildings is a limited one only because the factor of safety is not because of higher shear stress, and increased the ground subsidence factor is triggered because of the action of piping along a linear path (NSR 604) affecting from the top Sand Gully, Moulton Ridge, Slogher and Warden area.

4.5. Conclusion:

- i. The recent ground cracks are spread across a well-defined linear zone of 50-100 m wide along a linear zone trending NNEW - NW from westward to Sand Gully. The recent onset of ground subsidence is only confined phase in this linear zone, the extent boundary of which is close to the existing topography alignment.
- ii. The base of which from the new outcrop point (Photo 3.1) in the P' valley, Marwani is a clear evidence of the building of excess pore water pressure back of the face line of the

slope (from 1.24). The readily action of water indicates **the removal of their source character from the soil which debris mass at an active regime of piping**, which might have originated in the last action of subsidence or failure.

15. In addition to the above, the higher vertical load of the multi-layered level structures within a smaller area, resting over a heterogeneous collapse soil debris mass, caused the development of a higher amount of their mass on a slope, in contrast to the lowering of their strength of interfacial slope forming mass because of interaction by the available shallow subsurface patched groundmass. The above dynamic mass concentration lowered the factor of safety and triggered initial stages of slope failure in the form of developing large active ground cracks, subsidence, and even downward water-sliding of valley road blocks affecting the stability of superstructures.
16. **Observation of a sequence of recent ground cracks in a linear array between Marston and West Lane piers** here indicates that it is the result of a local linear phenomenon, therefore, the presence of a single deep-seated circular failure, which can cut through the entire hill cross between Avon and Marston (assumed some very narrow). The recent ground cracks with width $\approx 10\text{ mm}$ $\rightarrow 10\text{ cm}$ are following the sequence which indicate some detachment planes which are defined in relation to slope and together mass load (very close to the roadway alignment on the eastern side), are also not engulfing a large area, in other words of localised area. Moreover, it is a known fact that in such a heterogeneous slope forming material, the possibility of having a single large-scale deep detachment plane can easily develop. These observations, however, will be investigated in further detail, having detailed geological mapping on a 1:5000 scale by LSI subsequently.
17. Non-appearance of any new ground cracks for the last two weeks and a substantial reduction in the discharge of water from the newly appeared seepage point at P' colony Marston are very positive indications of the fact that piping action gets reduced to a great extent with time, and the pace of ground subsidence has also been controlled to a reasonable level so far, which is extremely favourable to the stability of groundmass.

5

5.8. STABILITY OF THE AREA AROUND NARSINGH MANGIR TEMPLE PRECINCT

A temple dedicated to Lord Hanuman (an incarnation of Lord Vishnu according to the Hindu religion scriptures) is believed to have been constructed by Adi Shankaracharya. The kind of worshipping established at this temple is believed to have been built during the reign of King Lalacharya Puka of Kantow in the 9th century (Wikipedia.org). Every year during the winter season when the temple at Shri Rudraiah ji is closed, the powers of Shri Rudraiah ji drive worship Lord Rudraiah ji at Narsingh temple at Indrakuh (Photo 5.1). Keeping in view the historical and cultural importance of this holy temple, the issues of stability in the temple precinct, surrounding area and the down slope are being discussed separately in this chapter.



Photo 5.1 View of the temple precinct



Photo 5.2 Entrance gate, Narsingh temple



Photo 5.3 Narayag temple, Narayag Monastery



Photo 5.4 View of Narayag Monastery

After the existence of ground subsidence at Indimath (see 2.1), concerns were raised on the stability of the area surrounding the temple precinct and the slopes below it. There were few media reports about some ground cracks in the area close to the Narayag Monastery. Q/G1 team visited the Narayag temple precinct and took pictures in the surrounding areas and also on the slopes below the temple area. Narayag temple (Lat. N 30° 33' 22.96" and Long. E 79° 33' 31.88") area is located in Lower Mustang ward of Fig. 2.1, Photo 5.3 of Indimath area and is located at an elevation of 3100 m. The temple precinct is on a level ground and the outer courtyard area has been covered with Katsura grass (Photo 5.3 and photo 5.4).

5.3 Site Observations:

The temple precinct is located on the land in the lower middle part of Indimath area. The area is occupied by cropland (see 2.1) which is loose soil and somewhat old. This flat piece of land is located just over the crest of an old landslide scar (Photo 5.3). However, no signs of distress or structural damage was noticed on the temple structures, its doors, or the outer courtyard area.



Photo 5.1 View of Narvaigh Maud through Maudfjella



Photo 5.6 View of Slopes below Maudfjella

Field inventory was taken in the downstream area of Narvaigh Maud and Maudfjella High road (Photo 5.5). It has been observed that the down slopes are deposited gneiss at an angle of 30° to 45° towards NNW and they are occupied by old slide debris material which is homogeneous in nature and consists of large size rock boulders embedded in soft to clay matrix (Photo 5.6). A prominent stratigraphic, trending N – S to NNW – SSE, disconformity from the higher reaches near the Auli area and passes close to the Narvaigh Maudfjella area, and finally joins Aivalikata river (Photo 5.5). Four wet areas have been observed on the flanks of this unit, on the down slopes below Narvaigh Maudfjella. Prima-facie, no signs of ground distress or deformation were observed on the down slopes of Narvaigh Maudfjella.

5.2 Geological Assessment

A longitudinal profile has been developed using Datas Data data with 10 m spatial resolution, along a line joining Aivalikata River at the bottom and Auli at the top (Fig 5.1). A cross-section of the same profile, around the Narvaigh Maudfjella area has been prepared (Fig. 5.2) for better appraisal. A close study of these two profiles reveals that there is a sharp break in topography, marked by a vertical steep face of about 5–6 m, which indicates the presence of a paleo landslide scar in this area. The height of big boulders in the down slope area (Photo 5.2 and Photo 5.6), is a testimony to this inference. During the field inventory in and around the Narvaigh Maudfjella complex and the surrounding area, prima-facie, signatures of recent ground distress (associated with the onset of 2021-22 FY) were not noticed and people were still residing in the hill slope in this area (Photo 5.1). The absence of signatures of ground distress, prima-facie, indicates that the

development area of Marleigh Mounds has assumed a certain degree of activity in the past few years. From here, the exposures of the polar thin bed (Plate 3.5) has also not been assumed on the ground.

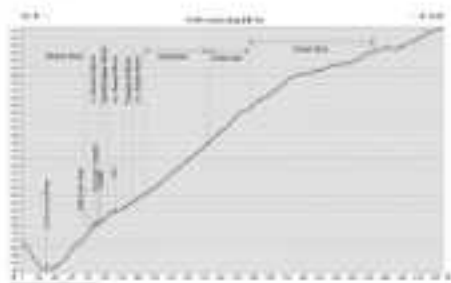


Figure 3.1.1. Profile tracing Marleigh Mounds Area

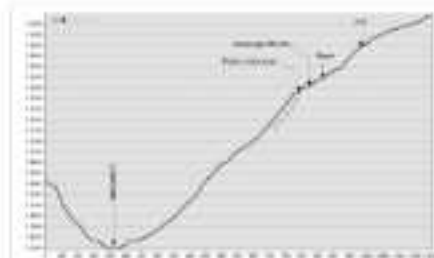


Figure 3.1.2. Elevation view of 1. Profile tracing Marleigh Mounds area, showing the position of the old old man

Some of the cracks that have been reported to the area around the sample complex are old cracks and are more or less related to structural defects. The reported crack on the storm outside for

simple process (Photo 5.7) is an opening along the construction joint which is confined to the lower part of the stair (i.e. the upper part of the stair, which is an older one, is unaffected and does not show any indications). Similarly, the reported crack on the side wall of the temple process (Photo 5.8) is an old one and you can see the paint and plaster inside the crack.



Photo 5.7 Opening of Construction joint outside. No major crack.



Photo 5.8 Deliberate collapse of an old boundary wall.



Photo 5.9 Opening of Construction joint along two different segments of walls.



Photo 5.10 Opening of Construction joint along two different segments of wall.

The fine cracks is reported on the two segments of walls as depicted in Photo 5.9 and photo 5.10 are at the junction of two segments which have different construction materials and signify a different time of construction. The issue is caused by the construction joint between the two segments of the wall as there is Photo 5.9 (see the pre-existing construction joints). In case the same segment is covered with current pouring, a fine crack would appear in this location after some time, which may

be suitable for a crack related with the ground settlement. It is interesting to note that one very old construction (purple line structure) just inside the Nanyang temple precinct, which has been constructed using rock blocks (Photo 2.11) and Photo 2.12) does not show any sort of structural damage/crack.



Photo 2.11: Old masonry structure close to Nanyang temple



Photo 2.12: Possible line of discontinuity due to a crack at Nanyang temple

However, if the surface is lateral along with mud or cement plaster, fine cracks parallel to block joints and construction joints would be seen after some time. The same has been depicted with the help of red lines as shown in Photo 2.12.

2.3 Conclusion:

Presently, the Nanyang temple precinct has not been found affected by the recent event of accidents reported on 11.01.2015. The temple structure and its precinct area are found to be found safe as observed during the current field exposures. There is an old slide scar before the area of Nanyang temple, however, as of now, any regression of the scar was not observed. The reported cracks, in the stone and walls in the outside area, are old ones and cracks are less pronounced. Plaster on wall existing in the building in the area adjoining the Nanyang temple which means that they have also not been affected by the recent event of 11.01.2015. No ground distress was or the settlement in the slope below the Nanyang temple area which indicates that presently the Nanyang temple precinct and its immediate surrounding area are geotechnically stable.

6.1. POSSIBILITY OF THE SEEPAGE FROM THE HRT OF TAPVAN VOLTAICAD HYDROPOWER PROJECT (TVHP) OF NTPC Ltd.

Geological Survey of India has been involved in the construction stage engineering Geological investigations of the Tapovan Voltacal Hydropower Project (TVHP) from 2001 to 2003. During the above period, GSI was involved in the 3D geological mapping of HRT from contour and Cross section. 3D geological logging of the powerhouse and 3D Geological logging of possible seepage areas which were carried out in parts. Reports on these investigations by GSI was already provided to NTPC Ltd.

During the preliminary ground geological assessment after the construction of ground network in Indkamth (after 02-04-2022) for aspects of any (flow-related) damages and possibilities of any seepage from the existing Head Race Tunnel (HRT) of the Tapovan Voltacal Hydropower Project (TVHP) from Tunnel Boring Machine (TBM) and used the CMT & Slant method (RMT) Tunnel were critically analysed. During this study, GSI also studied the details of various features and design features of HRT, the status of excavation in different faces, and data on seepage in HRT (has, etc., provided by the NTPC Ltd. on request from GSI).

Based on the detailed report provided officially by the NTPC authorities at Indkamth, on the Tapovan Voltacal Hydropower Project (TVHP) and based on the previous reports of GSI on the prevalent geomorphology and geology of the area and findings of the current field visits to and around Indkamth area, GSI submit the following, in connection with the above relevant issue:

6.2. Salient features of the project

Tapovan Voltacal Hydropower Project (TVHP) is a multi-faceted scheme that envisages the generation of 328 MW of hydropower through an underground powerhouse having four 41 / 42

turnover of 0.03 MW expenditure each. The Road Rank Tunnel (RRT) of TMMSP is about 12 km in length and 5.69 m in diameter (Annexure - 1 and Annexure - 2). As per the layout drawing (Annexure - 3) provided by NITC Ltd, the overall length of RRT is about 15.1775 km in which at the junction of TMM with RRT is at E1 + 797.15m. Out of the total length of about 12 km, nearly 3.3 km is being excavated through a drill & blast tunnel boring machine (TBM) which is a non-destructive method of tunneling according to blasting (Annexure - 4). The remaining 8.7 km length of RRT from the intake side is being excavated through Drill and Blast Method (DBM).

6.2. Geology along RRT alignment:

As per the geological section along RRT, as provided by NITC Ltd. (Annexure - 5), the RRT reach has been excavated through different kinds of major gneiss, quartzite, fine grained granite, and quartz mica gneiss whereas the DBM reach has been excavated through rocks like quartzite with bands of schist, schistose gneiss, major gneiss and a small patch (near the intake portal) through mica schists. The reach of RRT regarding basement schist approximation is 1.12 km away (Annexure - 6) and this stretch has been excavated through TBM only (no blasting).

6.3. Present status of excavation in RRT

The status of excavation of different faces of RRT, as received from NITC Ltd. (Annexure - 7), reveals that the face of RRT through TBM is now at 618.555 m, which is just beyond the location of Pimpri side (Annexure - 2 and Annexure - 3). This reach has been provided with the pre-cast concrete lining sections. It means that there is an unexcavated full reach of about 2.1 km ahead of the present TBM face (Annexure - 8). The RRT from the intake end has been excavated up to 1.08 km, after which there is an unexcavated reach of 2.57 m beyond which another 7.14 m stretch has been excavated. Hence it leaves another 0.75 m stretch that needs to be excavated through TBM. Thus adding up to the entire status of excavation through different faces, it is apparent that there is an unexcavated full reach of about 2.77 km separating the faces of the TBM and DBM stretch (Annexure - 9).

6.4. Seepage from TBM Excavated Reach

The data on seepage through the RRT TBM stretch, as obtained from NITC Ltd. through email

about 18-04-2022 from AGM, Goolag, NIPC, Anuram, Anuram - 5 and Anuram - 1). The seepage at the TBM face at 5915 m is 0.007 liter per sec (LPS) or 0.02 liter per minute (LPM) and this has kept the same for the period November-2022 to 15-06-2023 (Anuram - 5). The seepage at 5.915 m is the point where the TBM must go ahead to 2.000 m (0.28 LPS or 4872 LPM). The total seepage through HBT at the inside TBM shaft portal (a) for the distribution of Anuram is 71.62 LPS or 2862 LPM. This is the cumulative seepage discharge from this entire 1.8 km long TBM excavated section so far. The TBM is stuck at 5915 m since 05-10-2020 (Anuram - 7).

6.2 Engineering Geological Approach

Apud Marai the above, three part main issues linked with the construction of the HBT of 151400 feet that have evolved are based on the technical facts provided by NIPC (Anuram - 2 to Anuram - 7) and the ground observations, as discussed below:

- 1) **Role of March in Tunnel:** As discussed earlier, the alignment of HBT (Anuram - 2) is located at a lateral distance of 1.1 km from the shaft spread of Anuram which is even more at Marai. This march has been excavated through HBT, and the TBM face is located far away from Anuram (a) for the layout provided by NIPC Ltd., Anuram - 2 and Anuram - 5) hence, prima-facie the very question of any blast-induced damages attributing to the current situation is very unlikely.
- 2) **Seepage through HBT towards the seepage point at Marai:** The HBT excavation is yet not completed and the tunnel is not carrying any water presently for power generation. The water level at the present TBM face at 5915 m is 1047.11m and the seepage discharge at this point is only 0.02 LPM (a) for the period about 18-03-2022 from AGM Goolag NIPC Ltd., Anuram - 5). The ground elevation at the location of the lower stretch of Sand Gull is 1012.81m, Maraiha High is 1015.04m and Sanghar is at 1016.00m. Hence it is evident that the lower level of HBT of the TBM face (which is spatially more than 4 km away from Anuram) is much below the ground levels of the locations of Sand Gull, Maraiha High, and Sanghar. Moreover, the seepage discharge at this underground point currently is only 0.02 LPM which is much less than the probable leakage of seepage observed at the surface on 01-03-2022 from the HBT of the Upper

Vidutragal Multi-purpose Project (TVMP) from the previously stated fact of 2012 up to the affected business of last month as mentioned above.

- 3) **Seepage in HRT from intake rod.** The status of excavated faces of HRT from the 1999 and 1/1 meter are ~4) reveals that there is no pronounced till mass of about 2.7 km width separating the two faces of 1999 and 1991; hence there is no possibility of a connecting the seepage line from intake, if any, to the 1991 excavated reach. Further, given facts, the seepage of water away from the URM excavated reach of 1991 to 1999 excavated reach leading to surface discharge also does not seem feasible.

7.6. CONCLUSIONS AND RECOMMENDATIONS

7.1. Terrain conditions and previous geology

- *Astermuth* and the surrounding area, like in the area Hunsrygen Fjell-Hvannfjell (HfH), bounded to the south by the main boundary fault (MBF) and to the north by the main Fungus-Suture Zone (FSZ). The study area at *Astermuth* lies on the hanging wall of a major regional tectonic discontinuity, i.e. the Main Central Thrust (MCT), passing about 1 km south of *Astermuth* zone.
- MCT juxtaposes the Indian to Main Himalayan rocks of the Central Crystalline Group over the Main Himalayan, metamorphosed rocks of the Garwal Group. The rocks around the *Astermuth* are related to the Main Formation of the Central Crystalline Group and they are represented by granitic gneiss, banded gneiss, quartz-mica schist, quartzite, and schistose quartzite.
- *Astermuth* zone however is situated over a thick sequence of sediments and old lacustrine deposits, which mainly consist of heavy, unconsolidated, and homogeneous clasts constituted by big boulders of granitic rocks embedded in a fine clay or silty matrix.

7.2. Heavy on subsidence in *Astermuth* and the well of copper mines

- The problem of ground subsidence in and around *Astermuth* zone had been noticed several times in the past by earlier workers of USI, MCI, which companies, constituted in 1976, also pointed out this problem and made certain recommendations. Therefore, the problems of sinking and subsidence in *Astermuth* zone are about four decades old as identified reported by many previous workers of USI.
- In July 2012 a team of experts had been constituted by the Government of Himachal Pradesh to study the reported instances of ground subsidence in the *Astermuth* area. However, the present report turned over to *Astermuth* pertains to the sudden aspect of ground subsidence

in different parts of Jaskiwade. Following a burst of subsurface water at a deep-flood escape point near SP-1, there is the early onset of (12-01-2023).

7.2. Present investigation and relevant field observations

- CBM team carried out the ground survey of the affected area and prepared an inventory of 66 ground cracks, out of which 19 cracks are old ones existing before (12-01-2023), while 47 are recent ones (as reported as is after (12-01-2023)). These recently-appeared ground cracks are mainly encountered in four numbers of wards (i.e. North Ganga, Mahadev High, Janghlan, and Mahamud). The majority of the recently appeared ground cracks are following a submergence line area trending N 60° – 50°.
- Majority of the ground cracks, irrespective of their orientation, are located in areas that are already populated and loaded with multi-storied buildings. The other areas where it is scarcity of its habitation has or has not been affected by the recent event (02-01-2023) of ground deformation and subsidence.
- The recent ground cracks are found spreading across a 30 × 50 m wide linear zone and trends N 60° – 50° from Mahamud to South Ganga, the western boundary of which is close to the existing railway alignment.
- Furthermore, the Narayagh waste dump project is safe and has not been affected by the recent event of (12-01-2023). There is an old slide area below the area of Narayagh waste, however, as of now, exposure of the scar was not observed. The reported cracks in the waste and soils in the peripheral part are old ones and more or less due to structural construction inadequacy; the ground distress was noticed on the down-slope of the Narayagh waste area which indicates the stability of this area.
- The existence of natural springs at a number of locations (i.e. zone indicated that the slope-forming heterogeneity (delta sandstone) is saturated with water at different levels. In addition, the saturation level of the soil mass, by other means such as surface precipitation or rainwater, results in the building up of interstitial pore pressure in the soil mass back of the slope face. This, in turn, gives rise to an adverse condition for the stability of the slope, as the water saturation decreases the shear strength of the slope-forming material.

7.4. Appraisal on possible connection of the current hazard event with the HST of Taprow-Volynskiy Hybrid Project

- Regarding the question of seepage from the HST of Taprow-Volynskiy Project, it is difficult to apprehend any such connection keeping in view the lateral distance of 1.3 km of HST from the urban spread of Jashimark which is even more at Marwan. From the tunnel traversed using the Tunnel Boring Machine (TBM), which does not generate any other natural damage. And the LHM face of the HST is located much away from Jashimark (9 km or more) from the urban spread of Jashimark. Hence, prime-fact for the very question of any flow-related damages attributing to the current situation is very unlikely, including tracing any plausible connection of seepage water from the HST to the current problem faced near JP (along seepage-outfall point at Marwan).
- Moreover, the event level of HST (1211731.37m) as the present TBM face at 8119912 m (which is far away from Jashimark) is much below the ground levels of the corresponding locations of Sogd Canal, Marwan Ditch, and Sanghar. Moreover, the seepage discharge at this underground point currently is only 1.52 LPM which is nowhere matching with the initially reported discharge of 100 LPM from the newly arranged surface seepage point at Marwan located at 12114677 m; near JP station. These facts, prime-fact, rule out any plausible linkage of seepage observed at surface on 02/01/2023 at Marwan with the seepage observed at the existing Head Race Tunnel (HRT) at the presently check-off level at 8115412 m.

7.5. Possible causes of the current hazard at Jashimark

- The burst of water from the new seepage point (Photo 5.5) at JP station, Marwan, is a clear indicator of build up of excessive pore water pressure back of the first face of the slope (Photo 5.5). The steady state of water indicates removal of fine matrix material from the old side ditch, thus an air-water mixture of piping, which might have triggered an accelerated action of subsidence or failure.
- The removal of fine material from the matrix or piping (by the shallow soil surface water flow line, or during the surface precipitation), in which the grain boundaries are weakened, results in creation of hollows or void spaces beneath the grain boundaries (Photo 5.16). With more and more removal of fine material, over a long period of time because of piping,

results in vertical settlement of joint-rock boulders under the influence of gravity. The settlement of such joint-rock boulders causes a sagging on the surface, particularly when the settlement of boulders is happening close to the ground profile. This ultimately is reflected in the form of ground fissures or cracks, which in the initial stages may be very fine, but with the continued process, they may develop progressively widening with time.

- In Indusmith area, it is also common to notice construction of such overtop houses over steps having thick breccia-like overburden material (Photo 4.14). Any such piping action or discolored stream can yield ground cracks that are secondary effect and can develop into piping cracks on building walls and floors in such a way, as observed during the recent event of subsidence at Indusmith.
- The heterogeneous old ridge defines a relatively poor in cohesive strength. The settlement of it by shallow soil surface water seepage is reducing the shear strength of the ventral bedrock. In addition, the extensive ventral bed, ventral by a dense network of water-eroded talusings, is a barren area only limited the factor of safety further because of higher shear stress, and actually accumulated the ground talus-defect factor is triggered because of action of piping along a linear path (NW-SE-NE) affecting from the top near Sand Gully, to Marwar Gully, Singdhar and Marwar areas on the down slope.

7.6. Insights about the source of shallow underground water and real impact from the studies by CGWB, and NGRI

- The missions of the study by CGWB have brought out a series of activities to a broad pattern between Marwar, Singdhar, and Sand Gully and corroborate the linear trend (NW-SE) which has also been pointed out by CGWB from ground observations, and the same trend of interaction with groundwater is found uniform with the linear array/rows of development of recent ground cracks. The direct findings both by UBI and CGWB, that corroborate that the locally-appeared seepage point at R² colony which had no initial discharge of dense (0.1-1 PM, as at 12.01.2023), is predominantly, connected to a perched water body that is located within the thick overburden/breccia material. The piping of this material by the rapidly-developed seepage has been instrumental in developing severe ground cracks/ subsidence, a second factor/real structure that are aligned along a linear array trending NW-SE, affecting parts of Marwar, Singdhar, Sand Gully, etc.
- The 15-Air studies by NGRI also corroborated the presence of a barren linear zone of highest

indicating that it lies on N4–S2 trend, which also corroborates GSI's ground observation, and also confirms CGO's observation on a linear fracture shadow (10–40 m) groundwater along the Elam, Singhar, and Marwar, which also showed a decrease in water content on weathering, and confirmed linkage of lowering of seepage water observed with time at the new seepage point at HP Colony in Marwar.

- The above vital instrumental surface and sub-surface observations by CGO and PGRI supports GSI's ground evidence, and also answers the possible queries of linking the status of seepage water that suddenly gushed out in the new holes of CD/CJRTD, and has been instrumental in developing correct models, and sometimes to some extent in a more broad sense affecting parts of the south of India situation.

7.2. Inferences on slope stability status and its possible future trend

- Occurrence of a sequence of recent ground cracks in a linear array between Marwar and Singhar from previous facts indicates that it is the result of a local phenomenon, and the chance of a single deep-seated circular failure, which can mobilize the entire hill mass between Sub and Marwar downstream water systems. The recent ground cracks with N45W–E35N, 1:4–8 trend are following the common trend, which indicates some detachment planes which are confined in Marwar High and Singhar area (and lying close to the roadway alignment) on the cracks width are also not exhibiting a large size, in other words of bedrock torn. Moreover, it is a narrow. For that in such a heterogeneous slope-forming material, the possibility of having a single large-scale deep detachment plane can rarely develop. These inferences, however, will be investigated in further detail during detailed geological mapping on a 1:500 scale by GSI subsequently.
- Non-appearance of any new ground cracks by the last two months and a substantial decrease in the discharge of water from the newly appeared seepage point at HP colony, whereas are very positive indicators of the fact that pumping action gets reduced to a great extent with time, and the pace of ground subsidence has also been arrested to a reasonable level in the

7.3. Suggested way forward and further investigation by GSI

- As already planned and discussed, GSI will be carrying out detailed geological mapping on a 1:500 scale of the area around bedrock, covering all wards and the area between Sub, at the top, to Nalmandla River in the bottom. This (survey) will be started from the date of

map of the contour plan of the area as a 1:5000 scale with a 2 m contour interval by the USGS/A. The detailed geological mapping will be carried out by a team of an geologist equipped with GPS, and Interval Laser Scanner (ILS) instruments for data and point cloud coverage of the area. During the geological mapping, the ratings of soils, water (shallow) resources, distribution of different types of depositing material, other geomorphic features, ground failure features (Rf and Rm), natural springs, and newly appeared collapse points will be observed and mapped. The samples of soil and undisturbed soil will be collected for the determination of geotechnical parameters. Longitudinal and cross-sectional profiles will be developed from the geological map and geological sections will be prepared. Based on the ground geological mapping and results of soil samples, slope stability to determine the detailed slope stability assessment of the entire area, along with a preparation of a 1:5000 scale landslide hazard map. In addition to the landslide hazard map, as an ultimate output, a landslide management map will also be prepared wherein the mapped area will be categorized into different vulnerability zones. The non-specific hazard assessment and risk evaluation will be brought out and accordingly some specific/non-specific mitigation measures will be suggested. It will help the administrative authorities in planning the future developmental activities in and around the landslide area, and also use such Geoinformation for implementing land use zoning regulations in landslide to ensure better stability and management in the future.

- It is also suggested that for the above comprehensive slope stability assessment as a 1:5000 scale, the data and reports of the geophysical surveys carried out by WGB, USGS/A, and Waka Institute, results of water sampling by ARI, and results of geotechnical investigations and testing being planned carried out by IIT Bombay, along with geo-locations of sample points may please be shared with GSI to expedite the study, and to avoid any duplication of work, and also for integration in its final report so that the overall mitigation and rehabilitation measures can be suggested more precisely.
- It is strongly advised to carry out ground-based systematic monitoring in different parts of landslide area in order to get a timely and accurate assessment of ground movement, if any, in different parts of landslide. There are required beyond the presently available results of remote sensing or GPR studies.

ACKNOWLEDGEMENT

- The authors express their gratitude to Dr. S. Raja for Director General, of the Geological Survey of India for his overall moral guidance and supervision and for providing the facilities to carry out the work. The authors would like to thank Dr. N. Narasimhan, Additional Director General and IAS, for providing necessary moral guidance and providing the facilities and other support to carry out this work. Discussions with Dr. Director General, M-F A, Dr. Sanjay Jain and IAS, M-F C, Dr. Subal Ghosh, and Director (Geology), Ministry of Mines (MoM) Dr. Pradyumn Singh and Asst. C. Director, Dr. Kamesh Chakraverty were immensely helpful and is sincerely acknowledged.
- The Corporation received from the officials of local Administrations, especially the IASDA, Secretary Dr. Raju Kumar Singh, Secretaries of IASDA, viz. Dr. Suman Sarin, Dr. Piyush Khandel, and the DM of Chandel District for their assistance and support. Discussions with Scientists of other Scientific organisations working there, especially for Prakash Kumar, Director, IIGW, Dr. Jyoti Prasad, of MGR, and Dr. H. P. Karmakar, CMR, Barabanki, Jharkhand, Vidhansabha, WB, are very interesting. Lastly, the co-operation of the innumerable local people received during the work is sincerely appreciated.

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[External] Re: Request for sharing data

Shumesh <SHUMESHKUMAR@NTPC.CO.IN>

Sat, 07/01/2024 10:22

to: CD MR Vinay <vgalgotra>

cc: Dharmendra Singh <DHARMENDRA@NTPC.CO.IN>; Anurag <ANURAG@NTPC.CO.IN>; Harman Bedi <HARMANBEDI@NTPC.CO.IN>; RANJAN DAKSHINAYAT <RANJANDAKSHINAYAT@NTPC.CO.IN>; HARSH BHARGAVA <harshbhargava@gmail.com>; Harishchuraji <harishchuraji@gmail.com>; Chandra Char Teja <CDCHARTAJA@NTPC.CO.IN>; RAJENDRA KADAM <RAJENDRAKADAM@NTPC.CO.IN>

§ 4 documents (pdf)

File 1, 2, 3, 4 - MR-D-00534.pdf, Form 1, updated position of the HRT face from intake, from Chormi adit and from TBM excavation, Hatch.pdf, Form 2, Geological section along HRT.pdf.

सविनम्र अभिवादन,

Please find attached the following as desired:-

- 1) Final layout of the project clearly depicting the alignment of HRT
- 2) Updated position of the HRT face from intake, from Chormi adit and from TBM excavation.
- 3) Geological section along HRT.
- 4) Data on the seepage in HRT at the present TBM face - Seepage is 4 litres/minute from TBM Rock at present.

सहृदय (Regards),

शुभेश कुमार / Shumesh Kumar

अदन सहस्रबंधु (यु. विज्ञान) / AQAM (Geology)
T-1400
एनटीपीसी लिमिटेड, NTPC Limited
8620000471

-हिन्दी टैर की एक सूच में मिलने वाली भाषा है-

From: RAJENDRA PRASAD <RAJENDRA@NTPC.CO.IN>

Sent: 14 January 2023 16:52

To: Chandra Dhar Tewari <CDT@NTPC.CO.IN>; Shumesh <SHUMESHKUMAR@NTPC.CO.IN>; Anubhav <ANUBHAV@NTPC.CO.IN>; NAVEEN CHANDRA RAJ <NAVEENCHANDRA@NTPC.CO.IN>

Subject: Fwd: Request for sharing data

PLEASE SEE ATT 1

ATTACH

THU JAN 12 2023

From: HOD:MR chod.n@nptcl.co.in

Sent: Friday, January 12, 2023 8:26:57 PM

To: RAJENDRA PRASAD <RAJENDRA@NTPC.CO.IN>

Subject: Fw: Request for sharing data

WARNING: This email has been sent from outside the Organisation. Unless you trust the sender, don't click links or open attachments as it may be a phishing email, which may steal your information and compromise your Company.

To:
 Dr R.P. Anubhav
 Chief General Manager
 NTPC,
 Raigarh, Jashimach

Re:

Director General GSI, Kolkata has deputed Dr. Harish Bahuguna, Director, GSI, as the nodal officer for the geological assessment of the ongoing problem of ground subsidence at Jashimach. He shall be carrying out field assessment from 14th to 16th January 2023. In this context it is requested to kindly provide him the following:

- 1) Final layout of the project clearly depicting the alignment of HRT
 - 2) Updated position of the HRT face from intake, from Chormi adit and from TBM excavation.
 - 3) Geological section along HRT
 - 4) Data on the seepage in HRT at the present TBM face, if any.
 - 5) Any other relevant data/information as required by Dr. Bahuguna.
- His contact number is 9566663887.

This issue is being monitored by the highest level of officials of Government of India.

10/10/2023

नमो, जी. निम्नलिखित/ N.M. Nimmavada
आयुक्त निदेशिका/AYUKTA NIDESHIKA/ADG & HAD
आयुक्त, कोलकाता/ GSI, NR, Lucknow

आयुक्त/ Ragsdale

निदेशिका/AYUKTA NIDESHIKA/ADG & HAD
आयुक्त, कोलकाता/ GSI, NR, Lucknow

From: HOD NR
Sent: Wednesday, January 11, 2023 2:50 PM
To: rajiv@rajiv.in
Cc: NATIONAL MISSION HEAD 4, DDO DOW
Subject: Request for sharing data

To:
Dr. R.P. Singh
Chief General Manager
T-400
NTPC, Raurkela, Jharkhand

Sr.

Director General, GSI, Kolkata has deputed Dr. Harish Bahuguna, Director, GSI, as the nodal officer for the geological assessment of the growing problem of ground subsidence at Jharkhand. He shall be carrying out field assessment from 14th to 16th January 2023. In this context it is requested to kindly provide him the following:

- 1) Final layout of the project clearly depicting the alignment of HRT
- 2) Updated position of the HRT face from intake, from Chormi edit and from TBM excavation.
- 3) Geological section along HRT
- 4) Data on the seepage in HRT at the present TBM face, if any.
- 5) Any other relevant data/information as required by Dr. Bahuguna

His contact number is 9866003887.

This issue is being monitored by the highest level of officials of Government of India.

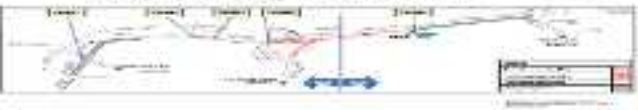
10/10/2004

लेनो जी. गिगनोरे, JMW International
अध्यक्ष, एडिनिंग्स एंड गिगनोरे, A/DG & HoD
मार्ग, कलेक्ट, OSI, NE, Lucknow

1922

2000

Figure 1: (a) Schematic diagram of the experimental setup for the study of the effect of the magnetic field on the growth of the dendrites. (b) Photograph of the experimental setup. (c) Photograph of the dendrites grown in the presence of the magnetic field. (d) Photograph of the dendrites grown in the absence of the magnetic field. (e) Photograph of the dendrites grown in the presence of the magnetic field. (f) Photograph of the dendrites grown in the absence of the magnetic field. (g) Photograph of the dendrites grown in the presence of the magnetic field. (h) Photograph of the dendrites grown in the absence of the magnetic field. (i) Photograph of the dendrites grown in the presence of the magnetic field. (j) Photograph of the dendrites grown in the absence of the magnetic field. (k) Photograph of the dendrites grown in the presence of the magnetic field. (l) Photograph of the dendrites grown in the absence of the magnetic field. (m) Photograph of the dendrites grown in the presence of the magnetic field. (n) Photograph of the dendrites grown in the absence of the magnetic field. (o) Photograph of the dendrites grown in the presence of the magnetic field. (p) Photograph of the dendrites grown in the absence of the magnetic field. (q) Photograph of the dendrites grown in the presence of the magnetic field. (r) Photograph of the dendrites grown in the absence of the magnetic field. (s) Photograph of the dendrites grown in the presence of the magnetic field. (t) Photograph of the dendrites grown in the absence of the magnetic field. (u) Photograph of the dendrites grown in the presence of the magnetic field. (v) Photograph of the dendrites grown in the absence of the magnetic field. (w) Photograph of the dendrites grown in the presence of the magnetic field. (x) Photograph of the dendrites grown in the absence of the magnetic field. (y) Photograph of the dendrites grown in the presence of the magnetic field. (z) Photograph of the dendrites grown in the absence of the magnetic field.



[External] [Re:] External [Re: Request for sharing data

Subject: Request for sharing data of TBM and HRT

Dear Sir,

I am a student of M.Tech in Environmental Engineering at IIT Guwahati.

I am currently working on a project titled "Design and Development of a TBM for the construction of a tunnel for the purpose of water supply in the city of Guwahati." I am looking for data related to the TBM and HRT for the purpose of my project. I am looking for data related to the TBM and HRT for the purpose of my project.

I am looking for data related to the TBM and HRT for the purpose of my project.

I am looking for data related to the TBM and HRT for the purpose of my project.

Request for sharing data

I am looking for data related to the TBM and HRT for the purpose of my project.

I am looking for data related to the TBM and HRT for the purpose of my project.

- The most level of HRT at the present TBM face at RD 52.10 m is 1.121 m.
- The most level of HRT at RD 52.10 m from TBM away gate is 1.121 m.
- The most level of HRT at RD 52.10 m from TBM away gate is 1.121 m.

I am looking for data related to the TBM and HRT for the purpose of my project.

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I am looking for data related to the TBM and HRT for the purpose of my project.

75th Azadi Ka Amrit Mahotsav



Subject: 75th Azadi Ka Amrit Mahotsav

Date: 11 January 2022 10:00

To: Shrikanth Choudhary, 75th Azadi Ka Amrit Mahotsav

Dr Chaman Singh, 75th Azadi Ka Amrit Mahotsav

75th Azadi Ka Amrit Mahotsav

75th Azadi Ka Amrit Mahotsav

Subject: 75th Azadi Ka Amrit Mahotsav

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Dr Chaman Singh
75th Azadi Ka Amrit Mahotsav

Dr Chaman Singh
75th Azadi Ka Amrit Mahotsav

Subject: 75th Azadi Ka Amrit Mahotsav

Date: 11 January 2022 10:00

To: Shrikanth Choudhary

Dr Chaman Singh, 75th Azadi Ka Amrit Mahotsav

Subject: 75th Azadi Ka Amrit Mahotsav

Dr Chaman Singh
75th Azadi Ka Amrit Mahotsav

Dr Chaman Singh
75th Azadi Ka Amrit Mahotsav

Abstract

See [Lecture 10](#) for more details.

Author: KODI M. CHEN (862) 221-1111
 Date: Friday, January 02, 2015 11:30 AM
 To: ALLISON.MALLON@NIAHVALE.COM
 Subject: RE: 441-4010-0107010000

[illegible]

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Dr Peter Schmitt-Nowara, who has headed the Central Fisheries Division since 2001, is the main officer for the ecological assessment of the ongoing problem of ground siltation at Lindeman. He shall be carrying out field assessments from 5.00 to 5.05 pm on 28th January 2023, in this respect it is requested to kindly provide him the following:

- 1) General layout of the project clearly depicting the alignment of MIT
 - 2) Detailed position of the MIT bus from station from District additional from TSPU station
 - 3) Geological section along MIT
 - 4) Data on the average of MIT in the present TSPU zone, if any
 - 5) Any other relevant data/information as required by Dr. Sahaguna
- We request you to a Sincerely,

The issue is being monitored by the highest level of officials of Government of India.

संपर्क के लिए: 011-26109600
www.mpsindia.org या info@mpsindia.org और admission@mpsindia.org
एडमिशन परीक्षा: 011-26109600

www.24x7news.com

Haupt- & Nebengeschäfte
 für den Handel mit Waren aller Art
 in der Stadt und im Lande
 1890 bis 1900

From: 60908
To: Wednesday, January 11, 2006 11:57 AM
Cc: ryan@geopoint.com
Subject: RE: GEOPOL - 01-11-06 @ 11:57 AM

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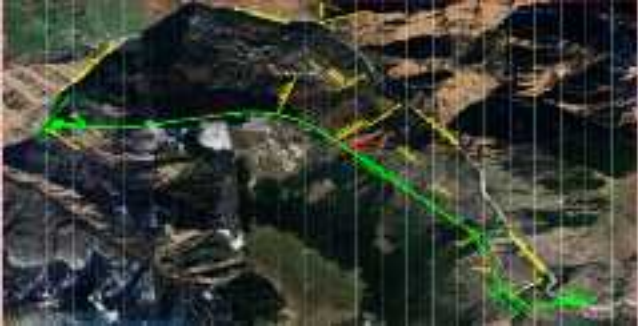
Director General (D), Odisha has deployed Dr. Harish Bahuguna, Director (D), as the nodal officer for the geological assessment of the mining position of ground subsidence at cuttack, to shall be carrying out field assessment from 04th to 20th January 2015. In this context it is requested to kindly provide the following:

- 1) Final layout of the project clearly depicting the alignment of HRT
- 2) Location position of the HRT from mainline, from District station & from Tiki station.
- 3) Geological section along HRT
- 4) Data on the proposed HRT as the present HRT line, if any.
- 5) Any other relevant data/information as required to do the work.

The issue is being monitored by the highest level of officials of government of India.
Yours sincerely,

Chief Engineer, JICA
and representative from JICA & JICA
4/4/14 to 14/01/15, Lucknow

1/14/15
1/14/15
1/14/15



[External]Re: [External]Re: [External]Re: Request for sharing data

Dharmendra Singh <DHARMENDRABIST@NTPC.CO.IN>

14/12/2022 10:41

to: HATTA, RAHUGURU <rahuguru@nptcl.co.in>, HOD 181 <hod181@nptcl.co.in>

cc: E.C. Pargaien <ECPARGAIEN@NTPC.CO.IN>, NARAIN CHANDRA RAO
 <NARAINCHANDRARA@NTPC.CO.IN>, Anshu <ANSHU@NTPC.CO.IN>, Bhushan
 <BHUSHAN@NTPC.CO.IN>, Mahesh Reddy <MAHESHREDDY@NTPC.CO.IN>, Chandu Char
 Tani <CTTANIR@NTPC.CO.IN>, RAJENDRA PRASAD <RAJENDRAPRA@NTPC.CO.IN>

महोदय/महोदया(Sir/Madam),

-The TBM reached at RD 5018m on 24/11/2019.
 -The excavation from 5018 was restarted on 6/03/2021 and got stuck at RD 5859m on 24/10/2022.
 -TBM restarted from RD 5859m on 03/05/2023 and got stuck at RD 5892 on 07/10/2023.
 -TBM restarted from RD 5892 on 21/09/2023 and got stuck at RD 5915 on 05/10/2023.
 Since 05/10/2023 TBM is stuck up till date at this position.

The Seepage of approximately 0.067 ltr/second has been observed from the TBM face at RD 5915m for the period November 2022 to 02-01-2023.

सुधु(Regards),

D S Bhat
 AGM (Geology), TVHFF
 अपरमहाप्रबन्धक(भूगोल)
 एनटीएस लिमिटेड-NTPC Limited
 885091346

From: HARISH BANSUNA <harish@gsi.gov.in>

Sent: 20 January 2023 17:34

To: Shuneech <SHUNEECH@NTPC.CO.IN>; HOD HR <hod.hr@gsi.gov.in>

Cc: G.C.Pangalen <GCP@GSI@NTPC.CO.IN>; Dharmendra Singh

<DHARMENDRASINGH@NTPC.CO.IN>; MANGEN CHANDRA RANT

<MANGENCHANDRAPANT@NTPC.CO.IN>; Anubhav <ANUBHAV@NTPC.CO.IN>; Mammoet Bedi

<MAMMOETBEDI@NTPC.CO.IN>; Chandra Dhar Tewari <CDT@NTPC.CO.IN>; RAJENDRA

PRASAD <RPAH@NTPC.CO.IN>

Subject: Re: [External] Re: [External] Re: Request for sharing data

CAUTION: This email has been sent from outside the Organisation. Unless you trust the sender, don't click links or open attachments as it may be a phishing email, which can steal your information and compromise your Computer.

Sir

Kindly arrange to provide the data when the excavation of TBM reach between RD 5015m and RD 5016m was undertaken and since when the TBM is stuck at RD 5015m.

Also please share seepage data from the TBM face at RD 5015m for the period

November 2022 to 02-01-2023.

Please treat this as Most Urgent.

With regards,

(Dr. Harish Bansuna)

Director Engineering Geology Division & Head of Office

Geological Survey of India

Ministry of Mines, Govt. of India

UT: J&K, UT: Ladakh, Jammu

From: HARISH BANSUNA

Sent: 19 January 2023 10:58:52

To: Shuneech; HOD HR

Cc: G.C.Pangalen; Dharmendra Singh; MANGEN CHANDRA RANT; Anubhav; Mammoet Bedi;

Chandra Dhar Tewari; RAJENDRA PRASAD

Subject: Re: [External] Re: [External] Re: Request for sharing data

Sir

It is requested to provide the Geophysical Profile along HRT alignment at the

With regards, Sir/Madam,

(Dr. Harish Bajaj)
 Director Engineering Geology Division & Head of Office
 Geological Survey of India
 Ministry of Mines, Govt. of India
 U.I. J&L, U.I. Ladakh, Jammu.

From: HARISH BAJAJ@GSI

Sent: 18 January 2024 09:02:09

To: Bhuvnesh; HOD NR

Cc: G.C.Pargalen; Dharmendra Singh; NAVJYOT CHANDRA PANT; Anushay; Manmeet Badi;
 Chandra Dhar Tewari; RAJENDRA PRASAD

Subject: Re: [External] Re: [External] Re: Request for sharing data

Thank you sir I acknowledge the receipt of the information.

From: Bhuvnesh <BHUVNESH@GSI> [mailto:PC.CO.IN]

Sent: 18 January 2024 15:05:08

To: HARISH BAJAJ@GSI; HOD NR

Cc: G.C.Pargalen; Dharmendra Singh; NAVJYOT CHANDRA PANT; Anushay; Manmeet Badi;
 Chandra Dhar Tewari; RAJENDRA PRASAD

Subject: [External] Re: [External] Re: Request for sharing data

महोदय/महोदया(Sir/Madam),

Point wise reply to the queries vide GSI trailing mail are as under:-

1.The invert levels of HRT, as desired, are given below:-

- The invert level of HRT at the present TBM face RD 5815m – EL:1751.153m.
- The invert level of HRT at RD 3016m from TBM entry point – EL: 1720.9817 m.
- The invert level at TBM entry, RD 0.0m, point- EL: 1704.00 m.

2. Regarding 3d Geological logs/ face logs of TBM excavated reach, it is submitted that, TVHPP TBM is a double shield TBM and therefore it is not possible to prepare the '3d Geological logs/ face logs of TBM excavated reach', since excavated rock surfaces are not visible.

3. Regarding a plan of HRT alignment on SOI toposheet indicating the horizontal distance of HRT from the urban sprawl of Joshimath, it is submitted that-

HRT alignment on 1:50000 SOI toposheet is not available, however only strip survey along HRT alignment was carried out on 1:10000 scale, as per DPR requirements, which does not cover Joshimath area.

However, to understand the horizontal distance of HRT from the urban sprawl of Joshimath, HRT alignment is tentatively marked on latest Google image and distance from HRT are indicated in attached Fig 1.

4. The seepage/discharge data from different reaches of HRT TBM reach measured on 17-01-2023 are as under-

- i) Seepage at the present TBM face at RD 5015m is 0.007 Litres/Sec.
- ii) Cumulative Tunnel Discharge measured at Ring no. 1819(RD 2672m BPI Downstream) – 68.20 Litres/Sec.
- iii) Cumulative Total Discharge measured outside TBM Adit Portal is 71.62 Litres/Sec.

सदर[Regards],

धुनेश कुमार /Dhuvnesh Kumar

अवर-महाप्रबंधक (भू-विज्ञान) / AGM (Geology)

Typhat

एनटीवीसी लिमिटेड/WPPC Limited

8650991473

जिन्ही देश को रक चुन में पिरने वाली पाया है



From: HARISH BHARGUJA <h.bharguja@gsi.gov.in>

Sent: 16 January 2024 20:00

To: Bhuvnesh <BHUVNESHKUMAR@NTFC.CO.IN>; HOD NR <hodnr@gsi.gov.in>

Cc: Dharmendra Singh <DHARMENDRASGT@NTFC.CO.IN>; Anushay

<ANUSHAY@NTFC.CO.IN>; Manmeet Bedi <MANMEETBEDI@NTFC.CO.IN>; NAVEEN CHANDRA

KANT <KUNDEENCHANDRAKANT@NTFC.CO.IN>; Chandra Shekhar Tewari

<CETWARI@NTFC.CO.IN>; RAJENDRA PRASAD <RPSHERWAR@NTFC.CO.IN>

Subject: Re: [External] Re: Request for sharing data

CAUTION: This email has been sent from outside the Organization. Unless you trust the sender, don't click or open attachments as it may be a phishing email, which can steal your information and compromise your Computer.

Dr

Please take reference of my trailing mail. It is requested to kindly provide the desired data at the earliest.
With regards,

Dr. Harish (Bharguja)

Director Engineering Geology Division & Head of Office

Geological Survey of India

Ministry of Mines, Govt. of India

UT, J&K, UT Ladakh, Jammu.

From: HARISH BAHUGUNA

Sent: 15 January 2023 09:47:44

To: Bhupesh; HOD NR

Cc: Dhanendra Singh; Anubhai; Manmeet Bedi; NAVJEN CHANDRA PRAT; Chandra Char

Tewari; RAJENDRA PRASAD

Subject: Re: [External] Re: Request for sharing data

Dr

Thank you for sharing the drawings. After studying them it is requested to kindly provide the following:

1. The initial level of HRT at the present TBM face, at RD 1078m from TBM entry point and at TBM entry point.
 2. 3d Geological logs/ face logs of TBM excavated rock.
 3. A plan of HRT alignment on SDI topsheet indicating the horizontal distance of HRT from the urban stretch of Jashmath.
 4. The seepage/discharge data from different location of HRT TBM shaft.
- With regards,

(Dr. Harish Bahuguna)

Director Engineering Geology Division & Head of Office

Geological Survey of India

Ministry of Mines, Govt. of India

UT, J&K, UT Ladakh, Jammu.

From: Bhupesh <BHUPESH@NESHUMAR@NTFC.CO.IN>

Sent: 14 January 2023 12:21:02

To: HOD NR

Cc: Dhanendra Singh; Anubhai; Manmeet Bedi; NAVJEN CHANDRA PRAT; HARISH

BHAGUNIA; harishbahuguna@gmail.com; Chandra Char Tewari; RAJENDRA PRASAD

Subject: [External] Re: Request for sharing data

प्रेमचन्द्रप्रसाद(Private)

Please find attached the following as desired-

- 1) Final layout of the project clearly depicting the alignment of HRT.
- 2) Updated position of the HRT face from intake, from Chornai addt and from TBM excavation.
- 3) Geological section along HRT.
- 4) Data on the seepage in HRT at the present TBM face - Seepage is 4 Litres/Minute from TBM Face at present.

साहब/Regard(s),

भुवनेश कुमार /Bhuvnesh Kumar

अपर महाप्रबंधक (भू विज्ञान) / AGM (Geology)

NT/497

एनटीपीसी लिमिटेड/NTPC Limited

9850921471

"हिन्दी देश को एक दूर में पिरोने वाली भाषा है"



From: RAJENDRA PRASAD <RPAWPRASAD@NTPC.CO.IN>

Sent: 14 January 2023 16:52

To: Chandr Char Tavian <CDTNAAR@NTPC.CO.IN>; Bhuvnesh

<BHUVNESHU/MAR@NTPC.CO.IN>; Anubhav <ANUBHAV@NTPC.CO.IN>; NAVEEN CHANDRA

PANT <NAVEENDCHANDRAPANT@NTPC.CO.IN>

Subject: Field Request for sharing data

Please see for me

Shrihar

Get Outlook for GSI

From: KOD MR rhod.m@gmail.com
Sent: Friday, January 12, 2023 4:26:57 PM
To: RAJENDRA PRASAD <RPR@RPRMAIL.NTPC.CO.IN>
Subject: Fw: Request for sharing data

CAUTION: This Email has been sent from outside the Organisation. Unless you trust the sender, don't click links or open attachments as it may be a Phishing email, which can steal your information and compromise your Company.

To,
Shri R.P. Ashwari
Chief General Manager
TUM-22
NTPC, Faridkot, Jashmath

Sr,

Director General GSI, Kolkata has deputed Dr. Harish Bahuguna, Director, GSI, as the nodal officer for the geological assessment of the ensuing problem of ground subsidence at Jashmath. He shall be carrying out field assessment from 14th to 28th January 2023. In this context it is requested to kindly provide him the following:

- 1) Final layout of the project clearly depicting the alignment of HRT
 - 2) updated position of the HRT face from intake, from Chrome add and from TBM excavation.
 - 3) Geological section along HRT
 - 4) Data on the seepage in HRT at the present TBM face, if any.
 - 5) Any other relevant data/information as required by Dr. Bahuguna
- His contact number is 956555387

This issue is being monitored by the highest level of officials of Government of India.
With regards,

प्रेमेश जी तिलवधेरे / P.V. TILWADHARE

mailto:rahuln@nptcl.co.in

अन सहायिका एडिभि-सयडर ADO & HoD

भा.पु.क., उ००११ / GSI, NR, Lucknow

सयड / Regard

भेदसक सी.ड.ड. Director, GSI

दुनी अन सहायिका एडिभि-सयडर (अडि ADO & HoD)

भा.पु.क., उ००११ / GSI, NR, Lucknow

From: HOD Nil

Sent: Wednesday, January 11, 2023 2:50 PM

To: rahuln@nptcl.in

Cc: NATIONAL MISSION HEAD 4; DGG DGM

Subject: Request for sharing data

To,

Shri R.P. Akhwar

Chief General Manager

TU-077

NTPC, Navigram, Joshimath

Sir,

Director General GSI, Kolkata has deputed Dr. Harish Bahuguna, Director, GSI, as the nodal officer for the geological assessment of the ensuing problem of ground subsidence at Joshimath. He shall be carrying out field assessment from 14th to 20th January 2023. In this context it is requested to kindly provide him the following

- 1) Final layout of the project clearly depicting the alignment of HRT
 - 2) Updated position of the HRT face from intake, from Chorro addt and from TBM excavation.
 - 3) Geological section along HRT
 - 4) Data on the seepage in HRT at the present TBM face, if any.
 - 5) Any other relevant data/information as required by Dr. Bahuguna
- My contact number is 956555327.

This issue is being monitored by the highest level of officials of Government of India.
With regards,

नरेन्द्र बी. निठनावरे / N V Nitaware
अणु महाविद्यालय एवं विभागध्यक्ष / ADO & HoD
भा.भू.उ. लक्ष्मीनगर / GSI, NR, Lucknow

